

### THE CITY OF ST. PETERSBURG, FLORIDA

PLANNING AND DEVELOPMENT SERVICES DEPARTMENT
URBAN PLANNING AND HISTORIC PRESERVATION DIVISION

### STAFF REPORT

Community Planning and Preservation Commission Certificate of Appropriateness Request

Report to the Community Planning and Preservation Commission from the Urban Planning and Historic Preservation Division, Planning and Development Services Department, for Public Hearing and Executive Action scheduled for **Tuesday**, **August 9**, **2022**, **beginning at 2:00 p.m.**, in Council Chambers of City Hall, 175 Fifth St. N., St. Petersburg, Florida. Everyone is encouraged to view the meetings on TV or online at <a href="https://www.stpete.org/connect\_with\_us/stpete\_tv.php">https://www.stpete.org/connect\_with\_us/stpete\_tv.php</a>.

According to Planning & Development Services Department records, no Commission member or his or her spouse has a direct or indirect ownership interest in real property located within 2,000 linear feet of real property contained with the application (measured in a straight line between the nearest points on the property lines). All other possible conflicts should be declared upon the announcement of the item.



Case No.: 22-90200063

REQUEST: Review of a Certificate of Appropriateness application for a two-story

garage with ADU at 2051 Burlington Ave N, a contributing property to

a local historic district.

ADDRESS: 2051 Burlington Avenue North

OWNER: Sean R Lefort

APPLICANT: Chad Holman, General Contractor

LOCAL LANDMARK: Kenwood Section - Southeast Kenwood Local Historic District (18-

90300001)

PARCEL ID NO.: 24-31-16-11808-011-0150 LEGAL DESCRIPTION: BRONX BLK 11, LOT 15

ZONING: NT-2

### Historical Context and Significance

The single-family residence at 2051 Burlington Ave N was constructed in 1925 by A.W. Bedford as a one-story, five-room bungalow with garage. In 1949, the garage building was added onto and converted into a one-story garage apartment. The property card indicates that the rear building (2051 ½ Burlington Ave N) was condemned with demolition pending, but is undated. The garage apartment building was documented in the 1994 FMSF along with the main residence, but appears to have been demolished in the mid-1990s. Two permits were issued in 1995 for demolition of the single-family residence and the detached garage building, but the main house was not demolished.

The subject property is representative of the 1920s Craftsman-style bungalow with front gable roof form with clipped gable ends, creating a distinct architectural feature.

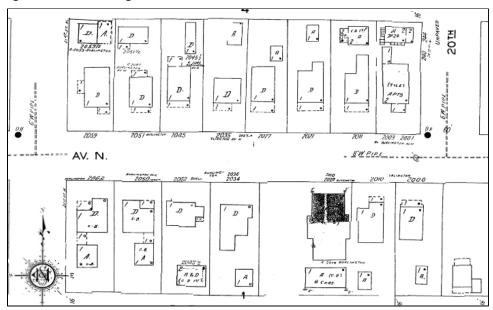


Figure 1: Sanborn Map of St. Petersburg updated 1951, Sheet 254 (partial), with subject property outlined in red.

### Project Description and Review

Application No. 22-90200063 (Appendix A) proposes the construction of a two-story garage apartment in the rear yard with the following characteristics:

- 576 square feet of living space above a 576 square foot garage,
- A two-story, front-gabled form with clipped ends featuring lap siding on the second floor and concrete block on first floor with a shingle roof,
  - The applicant states that they are willing to clad the entirety of the structure with lap siding.
- Two small gable overhangs to provide protection above doorways,
- One-over-one sash windows featuring traditional application of trim and installed with a recess in the wall plane, and
- An external stairway along the west side of the building to provide access to the ADU.

The new structure will be required to meet the building and design requirements for NT-2, parking, landscaping, and all other applicable land development requirements. The proposal has been reviewed by Development Review Services and found to meet Zoning requirements.

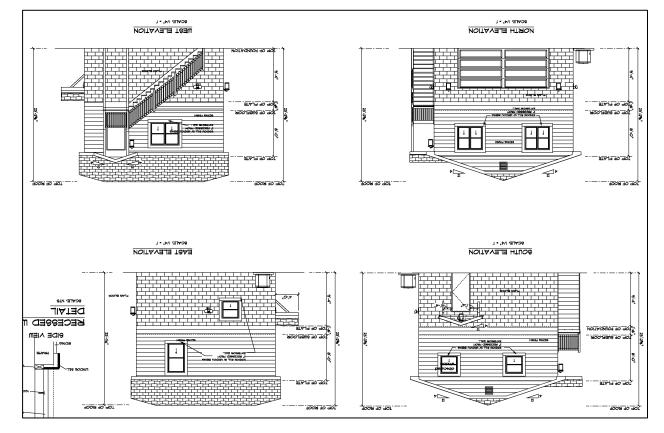


Figure 2: Proposed elevations from application 22-90200053.

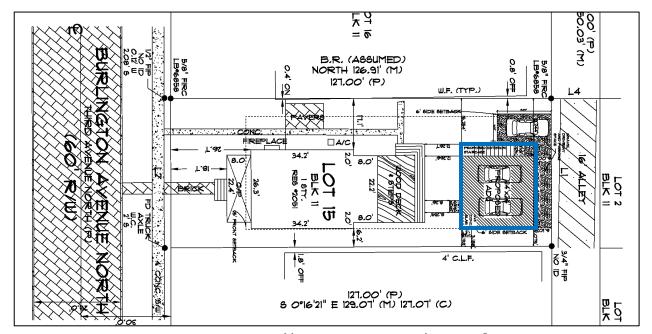


Figure 3: Proposed site plan from application 22-90200063. New ADU is outlined in blue.

General Criteria for Granting Certificates of Appropriateness and Staff Findings

1. The effect of the proposed work on the landmark or the property upon which such work is to be done.

Consistent

The subject district contains a mixture of one- and two-story accessory buildings. The proposal's height, scale, and materials are consistent with existing contributing buildings in the district and within the block.



Figure 4: Photographs taken from alleyway between the 2000 block of Burlington Ave N and 3<sup>rd</sup> Ave N that show several two-story garage apartment buildings.

CPPC Case No.: 22-90200064/22-90200065

2. The relationship between such work and other structures on the landmark site or other property in the historic district.

Consistent

The proposed project will be minimally visible and located on the rear of the subject parcel. Although it is taller than the primary residence, it is set well back on the parcel. It will have very little impact on the historic district.

3. The extent to which the historic, architectural, or archaeological significance, architectural style, design, arrangement, texture and materials of the local landmark or the property will be affected.

Consistent

The proposal will have little impact on the historic significance or architectural design of the property. The proposed garage apartment building utilizes architectural features, such as clipped gable ends, to match the main house.

4. Whether the denial of a Certificate of Appropriateness would deprive the property owner of reasonable beneficial use of his or her property.

Information
Not Provided

5. Whether the plans may be reasonably carried out by the applicant.

**Consistent** The proposed project appears to be appropriate under this criterion.

6. A COA for a noncontributing structure in a historic district shall be reviewed to determine whether the proposed work would negatively impact a contributing structure or the historic integrity of the district. Approval of a COA shall include any conditions necessary to mitigate or eliminate negative impacts.

Not

The house is a contributing resource to the subject district.

**Applicable** 

### Additional Guidelines for New Construction

In approving or denying applications for a COA for new construction (which includes additions to an existing structure), the Commission and the POD shall also use the following additional guidelines.

1. The height and scale of the proposed new construction shall be visually compatible with contributing resources in the district.

Consistent

The proposed new construction will be a two-story building located in the rear yard. There are numerous examples of contributing two-story garage apartment buildings in the subject district and on that specific block, as demonstrated in Figure 4.

2. The relationship of the width of the new construction to the height of the front elevation shall be visually compatible with contributing resources in the district.

Consistent

The proposed new construction will have a roof peak of 23 feet, 3 inches and a width of 24 feet. This is consistent with other garage apartments in the subject district.

3. The relationship of the width of the windows to the height of the windows in the new construction shall be visually compatible with contributing resources in the district.

**Consistent** Windows are vertically oriented, double sash, with paired windows filling larger openings. This is consistent with contributing properties in the district.

4. The relationship of solids and voids (which is the pattern or rhythm created by wall recesses, projections, and openings) in the front facade of a building shall be visually compatible with contributing resources in the district.

**Consistent** Windows are placed with a simple but orderly rhythm.

5. The relationship of the new construction to open space between it and adjoining buildings shall be visually compatible with contributing resources in the district.

**Consistent** The proposed garage apartment is placed near the rear property line and within required setbacks.

6. The relationship of the entrance and porch projections, and balconies to sidewalks of the new construction shall be visually compatible with contributing resources in the district.

**Consistent** The side staircase is typical to similar garage apartment buildings.

7. The relationship of the materials and texture of the facade of the new construction shall be visually compatible with the predominant materials used in contributing resources in the district.

### Consistent

The applicant is proposing a building with horizontal siding on the top floor, which matches the primary residence. The first floor is shown with concrete block. Evidence shows that many of the contributing two-story garage apartment buildings are painted masonry of plain or rusticated block on the first floor and horizontal oriented siding on the second floor. The change in materials between the first and second floor also helps to break up the massing of the building. This proposal is consistent with the other contributing resources.

While rusticated block that matches the main house would be the most appropriate material selection for the first floor, staff understands that rusticated block can be hard to source for new construction.

The applicant has stated that they are willing to utilize siding on both floors. Staff finds that simple concrete block on the first floor is more in keeping with other historic garage buildings, but the concrete block should have a painted finish to match the texture of the other garage buildings. Unpainted concrete block would create a contrasting texture and visual appearance.

8. The roof shape of the new construction shall be visually compatible with contributing resources in the district.

Consistent

The clipped gable end form was designed to match the primary residence on the property.

9. Appurtenances of the new construction such as walls, gates and fences, vegetation and landscape features, shall, if necessary, form cohesive walls of enclosures along a street, to ensure visual compatibility of the new construction with contributing resources in the district.

### Consistent

10. The mass of the new construction in relation to open spaces, the windows, door openings, porches and balconies shall be visually compatible with contributing resources in the district.

### Consistent

11. The new construction shall be visually compatible with contributing resources in the district in its orientation, flow, and directional character, whether this is the vertical, horizontal, or static character.

### Consistent

12. New construction shall not destroy historic materials that characterize the local landmark or contributing property to a local landmark district. The new construction shall be differentiated from the old and shall be compatible with the massing, size, scale, and architectural features to protect the historic integrity of the local landmark and its environment, or the local landmark district.

**Consistent** The proposed garage apartment will be placed in an open area, where the only structure is a non-historic metal shed.



Figure 5: Rear of subject property.

13. New construction shall be undertaken in such a manner that if removed in the future, the essential form and integrity of the local landmark and its environment would be unimpaired.

**Consistent** The proposed new construction will have no impact on the essential form and integrity of the local historic district.

### Summary of Findings

Staff evaluation yields a finding of the following criteria being met by the proposed project:

- General Criteria for Granting Certificates of Appropriateness: 4 of 5 relevant criteria satisfied.
- Addition Guidelines for New Construction: 13 of 13 criteria satisfied.

### Staff Recommendation

Based on a determination of general consistency with the requirements for demolition of historic resources defined by Chapter 16, City Code of Ordinances and based on the submitted information from the applicant, staff recommends that the Community Planning and Preservation Commission **approve** with conditions the Certificate of Appropriateness request for the new construction of a garage apartment at 2051 Burlington Ave N, , a contributing property to the Kenwood Section – Southeast Kenwood Local Historic District, with the following conditions of approval:

- 1. Windows and doors will be installed to be setback within the wall plane and feature a reveal of at least two inches.
- 2. Door style and design will need to be provided to staff prior to the issuance of building permits. If the doors feature a grille pattern, the door will need to have contoured, exterior three-dimensional muntins to reference historic muntin style.
- 3. The concrete block on the first floor will have a painted finish.
- 4. A historic preservation final inspection will be required.
- 5. All other necessary permits shall be obtained. Any additional work shall be presented to staff for determination of the necessity of additional COA approval.
- 6. This approval will be valid for 24 months from the date of this hearing, with an expiration date of August 9, 2024.

Report Prepared By:

**Kelly Perkins, Historic Preservationist II** 

Urban Planning and Historic Preservation Division Planning and Development Services Department

08.04.2022

Date

Report Approved By:

Duck A. Killow

08.04.2022

Derek S. Kilborn, Manager

Urban Planning and Historic Preservation Division Planning and Development Services Department

Date

### Appendix A:

Application No. 22-90200063



## CERTIFICATE OF APPROPRIATENESS

### **APPLICATION**

All applications are to be filled out completely and correctly. The application shall be submitted to the City of St. Petersburg's Planning and Development Services Department, located on the 8th floor of the Municipal Services Building, One Fourth Street North, St. Petersburg, Florida. Laura Duvekot, Historic Preservationist II, (727) 892-5451 or Laura. Duvekot@stpete.org

GENERAL INFORMATION	ON
2051 Burlington Ave. N., St. Petersburg, FL 33713	24-31-16-11808-011-0150
Property Address Historic Kenwood	Parcel Identification No.
Historic District / Landmark Name Sean R Lefort	Corresponding Permit Nos. 727-434-3472
Owner's Name 2051 Burlington Ave. N., St. Petersburg, FL 33713	Property Owner's Daytime Phone No. harriscn85@gmail.com
Owner's Address, City, State, Zip Code Chad Holman, General Contractor	Owner's Email 586-212-7645
Authorized Representative (Name & Title), if applicable 1211 1st Ave N., Suite 207, St. Petersburg, FL 33705	Representative's Daytime Phone No. c. holman@caskconstruction.com
Representative's Address, City, State, Zip Code	Representative's Email

	APPLICATION TYPE (Check applicable)				
	Addition	Window Replacement			
~	New Construction	Door Replacement			
	Demolition	Roof Replacement			
	Relocation	Mechanical (e.g. solar)			
	Other:				

TYPE OF WORK (Check applicable)
Repair Only
In-Kind Replacement
New Installation
Other:

### **AUTHORIZATION**

By signing this application, the applicant affirms that all information contained within this application packet has been read and that the information on this application represents an accurate description of the proposed work. The applicant certifies that the project described in this application, as detailed by the plans and specifications enclosed, will be constructed in exact accordance with aforesaid plans and specifications. Further, the applicant agrees to conform to all conditions of approval. It is understood that approval of this application by the Community Planning and Preservation Commission in no way constitutes approval of a building permit or other required City permit approvals. Filing an application does not guarantee approval.

- NOTES: 1) It is incumbent upon the applicant to submit correct information. Any misleading, deceptive, incomplete or incorrect information may invalidate your approval.
  - 2) To accept an agent's signature, a notarized letter of authorization from the property owner must accompany the application.

Signature of Owner:	Sean R Lefort	Date:	06/06/22
Signature of Representative:	Chad Holman	_ Date:	06/06/22



## CERTIFICATE OF APPROPRIATENESS

**APPLICATION** 

COA#

All applications are to be filled out completely and correctly. The application shall be submitted to the City of St. Petersburg's Planning and Development Services Department by emailing directly to Historic Preservationists Laura Duvekot (<u>Laura.Duvekot@stpete.org</u>) or Kelly Perkins (<u>Kelly.Perkins@stpete.org</u>).

### PROPOSED SCOPE OF WORK

Please provide a detailed description of the proposed work, organized according to the COA Matrix. Include information such as materials, location, square footage, etc. as applicable. Attach supplementary material as needed.

Building or Site Feature	Photo No.	Proposed Work
Detached Garage Unfinished	NO.	Build a new garage with an ADU above it. See attached plans with the detailed scope



View from alley. Currently now structure behind main house

View from Burlington Ave N., west side of house.



View from Burlington Ave N.

### 2051 BURLINGTON AVENUE NORTH ST. PETERSBURG, FL 33713

1. DWELLING FLOORS - 40 PSF LIVE LOAD

2. BALCONIES - 60 PSF LIVE LOAD 3. WALKWAYS - 80 PSF LIVE LOAD

4. ROOF OVER LIVING AREAS - 30 PSF LIVE LOAD, IT PSF (T PSF T/C & 10 PSF B/C) DEAD LOAD FOR SHINGLE ROOFS, 25 PSF (15 PSF T/C & 10 PSF B/C) DEAD LOAD FOR CONCRETE TILE ROOFS.

5. NET UPLIFT DEAD LOADS 10 PSF SHINGLE & 15 PSF TILE.

6. WIND - 145+ MPH, PER FBC UNLESS OTHERWISE NOTED.

T. CONSTRUCTION CATEGORY #2

8, RISK FACTOR #2

9, WIND EXPOSURE: "B" HEIGHT & EXPOSURE COEFFICIENT = 1.00

10, WIND IMPORTANCE FACTOR: IW = 1,00

II. INTERNAL PRESSURE COEFFICIENT PER ASCE 7-10 GCpi= +/- 0.18 (ENCLOSED)

THE FLORIDA BUILDING CODE 2020 7th, EDITION

2. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI318.99)

3. AMERICAN SOCIETY OF CIVIL ENGINEERS MIN. DESIGN LOADS FOR BUILDINGS & OTHER STRUCTURES (ASCE 7.05). 4. SPECIFICATION FOR THE DESIGN, FABRICATION & ERECTION OF STRUCTURAL STEEL FOR BUILDINGS (AISC 15th. EDITION)

5. "DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES" BY THE TRUSS PLATE INSTITUTE (TPI) 2014 EDITION

6. NFPA NATIONAL ELECTRICAL CODE (NEC.)

I. ALL CONNECTIONS SPECIFIED ARE PER SIMPSON CATALOG, CONTACT SOUTHERN DESIGN HOMES, Inc. FOR CONNECTOR SUBSTITUTIONS. 2, PRE-ENGINEERED TRUSSES TO BE SPACED @ 24" O.C. UNLESS OTHERWISE NOTED

3. IF THE TRUSS INFORMATION PROVIDED IN THESE CONSTRUCTION DOCUMENTS IS REVISED BY ANY OTHER PARTY OTHER THAN SOUTHERN DESIGN HOMES, Inc. FOR REVIEW, THE CLIENT IS RESPONSIBLE FOR ANY CONSTRUCTION COST RESULTING FROM TRUSS PACKAGE REVISIONS DEVELOPED BY OTHERS.

4. ALL TRUSSES TO BE DESIGNED PER ACCEPTABLE PROVISIONS OF THE FLORIDA BUILDING CODE AND APPLICABLE TIMBER CODES: TOP CHORD LL....20 psf

TOP CHORD DL..... 10 psf BOTTOM CHORD LL.....5 pef

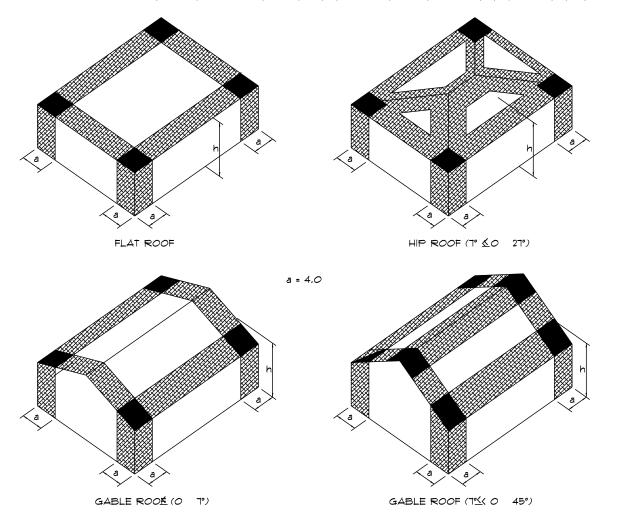
BOTTOM CHORD DL...10 psf

TRUSSES TO BE #2 SOUTHERN PINE OR BETTER

WIND.....SEE DESIGN LOADING ON THIS SHEET

5. PRE-ENGINEERED WOOD TRUSS ERECTOR IS REQUIRED TO HANDLE AND INSTALL TRUSSES PER MANUFACTURE'S INSTRUCTION, AS A MINIMUM, INSTALL BRACING IN ACCORDANCE WITH BCSI-1-03, AND LEAVE PERMANENTLY IN PLACE MOVE TO OPPOSITE SIDE OF CHORD IF NECESSARY DUE TO INTERFERENCE WITH SHEATHING OR CEILING MATERIALS, TOP AND BOTTOM CHORD MEMBERS OF THE PRE-ENGINEERED TRUSS SYSTEM FOR THE COMPLETE STRUCTURE ARE TO BE CONTINUOUSLY BRACED BY SHEATHING AND CEILING FINISHES, THE ROOF TRUSS SYSTEM IS NOT COMPLETED UNTIL ALL BRACING, SHEATHING AND FINISHES ARE PERMANENTLY ATTACHED.

### FIGURE R301.2(7) COMPONENT AND CLADDING LOADING PRESSURE ZONES



FOR 61: 1 FOOT = 304..8mm, 1 DEGREE = 0.0174 RAD. NOTES: a = 4 FEET IN ALL CASES.

1/ AREA OF PROPERTY\_\_\_\_\_\_6,323 SF. 2/ AREA OF EXISTING LIVING SPACE\_\_\_\_\_\_ 163 SF. 3/ ADDITION LIVING SPACE \_\_\_\_\_ 676 SF. 4/ IMPERVIOUS SURFACE AREA\_\_\_\_\_\_ 1,857 SF. 5/ IMPERVIOUS SURFACE RATIO: 1,857 SF/ 6323 SF = 29.4%

(Fb) = 1250 PSI AND A MODULUS OF ELASTCITY = 1,600,000 PSI (DOES NOT INCLUDE INTERIOR NON-LOAD BEARING STUD WALLS).

2, DESIGN, FABRICATE AND ERECT WOOD TRUSSES IN ACCORDANCE WITH THE "DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES" BY THE TRUSS PLATE INSTITUTE, 1985 ED. AND HIB 91 (IF APPLICABLE).

3. ALL EXPOSED WOOD OR WOOD IN CONTACT WITH EARTH OR CONCRETE SHALL BE PRESSURE TREATED.

4, ROOF SHEATHING, 1/2" C.D. GRADE PLYWOOD (OR 1/8" OSB), WHEN USING FIBERGLASS/ASPHALT SHINGLES OVER FELT 5. UNTREATED WOOD SHALL NOT BE IN DIRECT CONTACT WITH CONCRETE, SEAT PLATES SHALL BE PROVIDED AT BEARING LOCATIONS

WITHOUT WOODEN TOP PLATES. 6. WALL SHEATHINGS:

JUST FOR WALL INFILL, COORDINATE NAILING PATTERN WITH NOTE ON SHEET A-2.

7. FOLLOW ALL REQUIREMENTS OF THE 2020 FLORIDA BUILDING COSE AND CURRENT UPDATES, FOR ALL WOOD FRAMING INCLUDING BUT NOT LIMITED TO CONNECTIONS, BRACING, BRIDGING AND NAILING.

1. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI (UNLESS NOTED OTHERWISE) @ 28 DAYS.

2. REINFORCING BARS - ASTM A615 (GRADE 60)

3. WELDED WIRE FABRIC (WWF) - ASTM A185

4. DETAIL: REINFORCEMENT IN ACCORDANCE WITH ACI 315, WHERE APPLICABLE

5. CONCRETE COYERAGE OF REINFORCEMENT - FOOTING 3, BOTTOM 4 SIDES

6. EARTH SUPPORTING SLABS - (INCLUDING EXTERIOR WALK & DRIVE SLABS) 4" THICK

7. CONCRETING OPERATIONS SHALL COMPLY WITH ACI STANDARDS

8. LAP SPLICE SHALL BE AS FOLLOWS - \*5 REBAR 25", \*4 REBAR 20" , \*3 REBAR 15"

I, DESIGN & CONSTRUCTION SHALL CONFORM TO THE SPECIFICATION OTF THE NATIONAL CONCRETE MASONRY ASSOC, & ACI 530

2. MORTOR SHALL BE TYPE S or M

3. ALL BLOCK CELLS & CAVITIES BELOW GRADE SHALL BE FILLED WITH NON-SHRINKING CONCRETE

4. FILL CELLS W/(1) #5 REBAR SHALL BE LOCATED PER PLAN (6'-0" O.C.) AT EACH CORNER AND EACH SIDE OF OPENING GREATER THAN OR EQUAL TO 6'-0", FILL CELLS W/(2) #5 YERT, SHALL BNE LOCATED AT EACH SIDE OF OPENING 10'-0" AND GREATER, AT ALL GIRDER BEARING, INCLUDING HIP-SET GIRDER.

INSTALLATION OF ALL STEEL TO BE PREFORMED BY A QUALIFIED, LICENSED STEEL ERECTOR. 2. FABRICATE AND ERECT ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC "SPECIFICATIONS FOR THE DESIGN, FABRICATION

AND ERECTION OF STRUCTURAL STEEL FOR BUILDING.

3. STRUCTURAL STEEL TO BE ASTM A36 UNLESS OTHERWISE NOTED

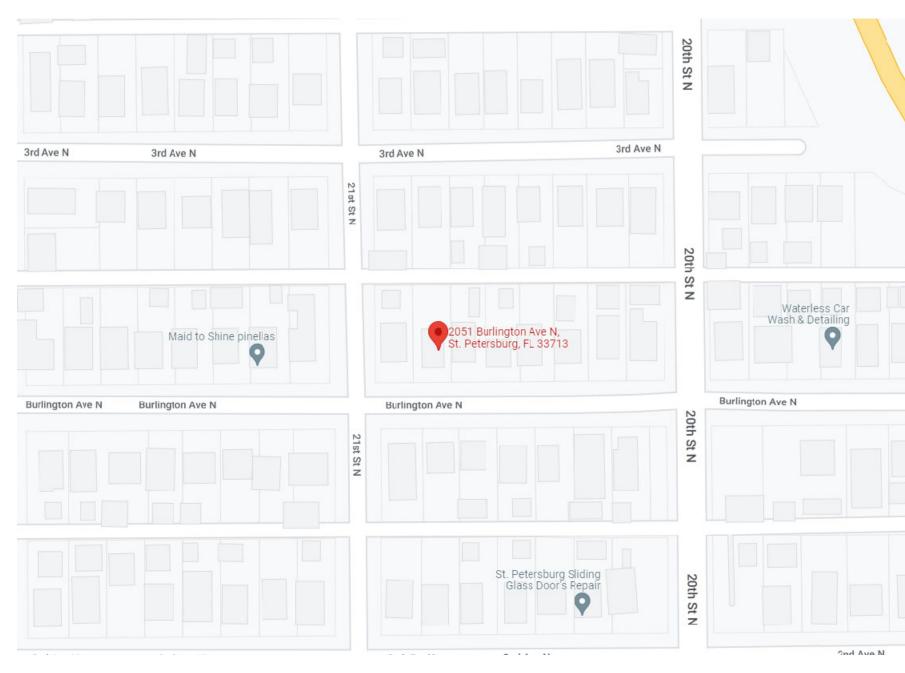
4. STEEL TUBING SHALL BE ASTM A500, GRADE "B".

5. WELDED CONNECTION, ETOXX ELECTRODES MIN. FILLET WELDS 3/16", AWS CERTIFIED WELDER.

6. WHERE STEEL BEAMS ARE CONTINUOUS OVER COLUMNS, PROVIDE WEB STIFFNER PLATES ON EACH SIDE OF THE WEB, OF A THICKNESS EQUAL TO BEAM FLANGE THICKNESS, LOCATED AT THE CENTER LINE OF THE TUBE COLUMN.

1. THE STEEL FRAME IS "NON-SELF SUPPORTING". ADEQUATE TEMPORARY SUPPORT SHALL BE PROVIDED BY THE CONTRACTOR UNTIL REQUIRED CONNECTIONS OR ELEMENTS ARE IN PLACE,

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ROOF UPLIFT CREE	OIT:		(psf)		
	EFFECTIVE	DESIGN WIN	D SDEED /	MDU	
		DESIGN WIN	D SPEED (	WPH)	
	WIND				
	AREA	-		116	(Vasd)
ZONE	(ft2)	30	35	( PER TABLE	R301.2.1.3)
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LOCATION MAP

### SHEET INDEX

COYER SHEET- GENERAL NOTES

EXISTING SITE PLAN

SHEET 2A PROPOSED SITE PLAN

SHEET 3 PROPOSED FLOORPLANS

SHEET 4 ELEVATIONS

SHEET 5 FOUNDATION AND FRAMING PLAN

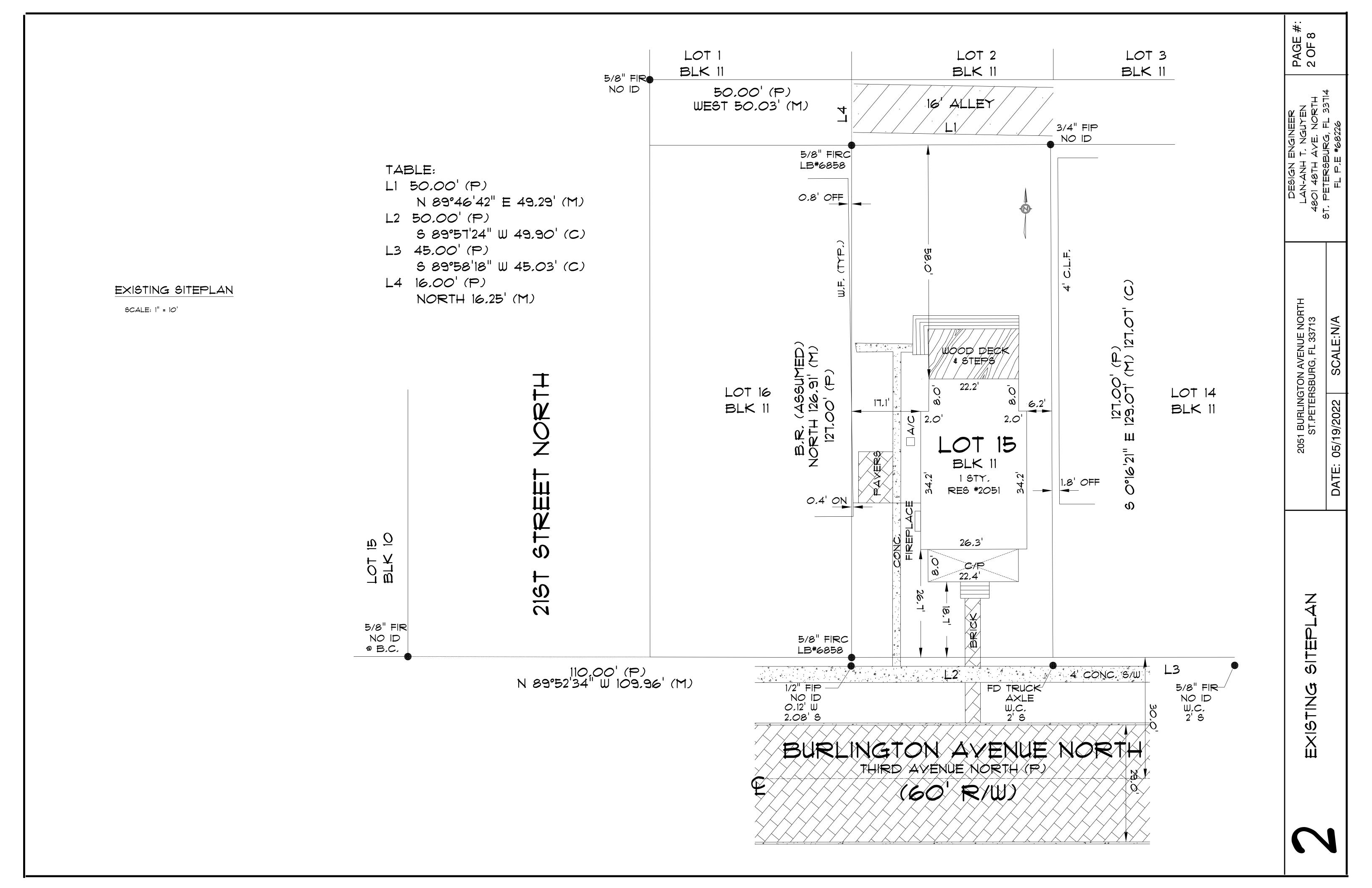
SHEET 6 ELECTRICAL PLAN

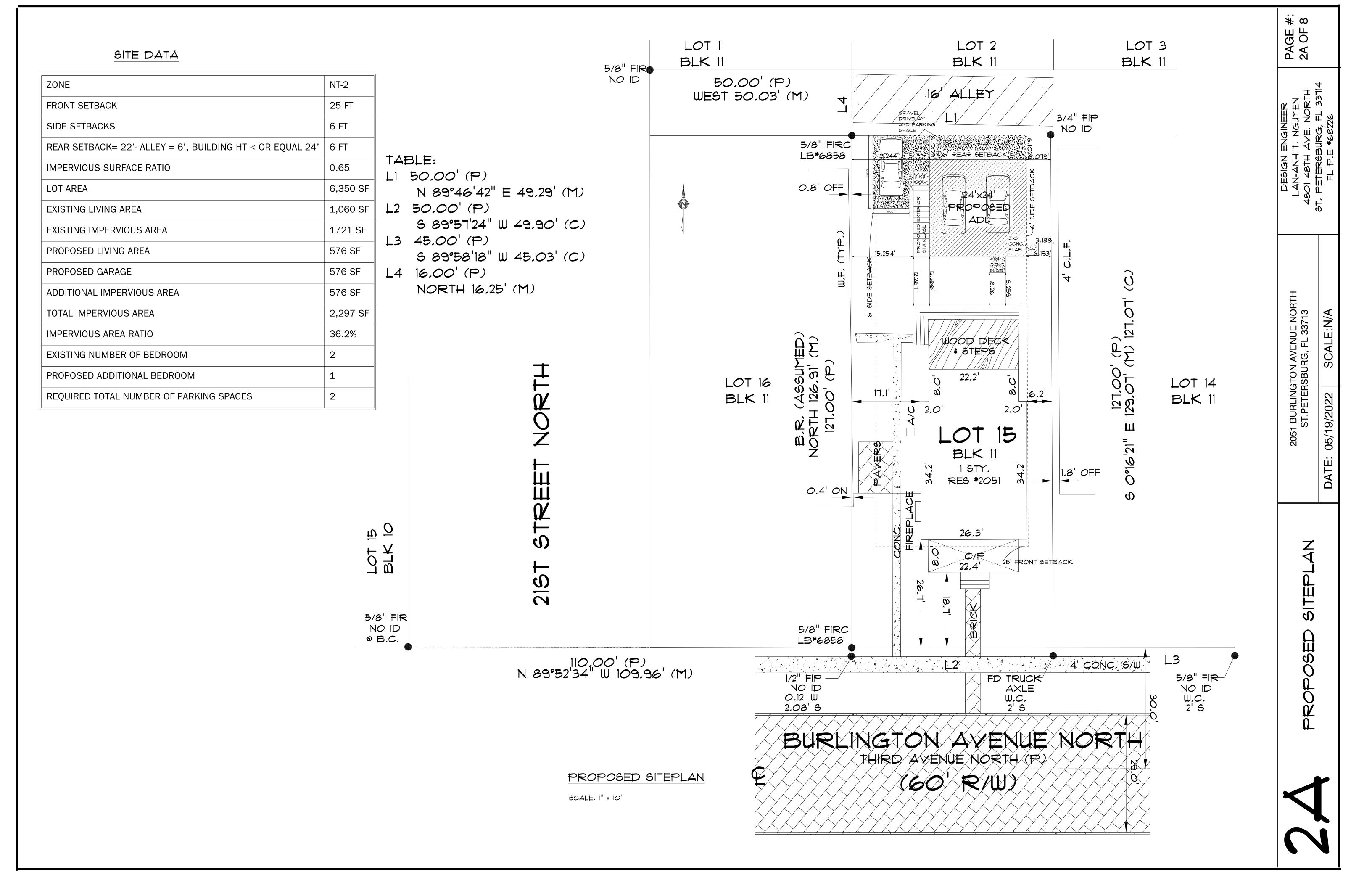
SHEET T DETAILS

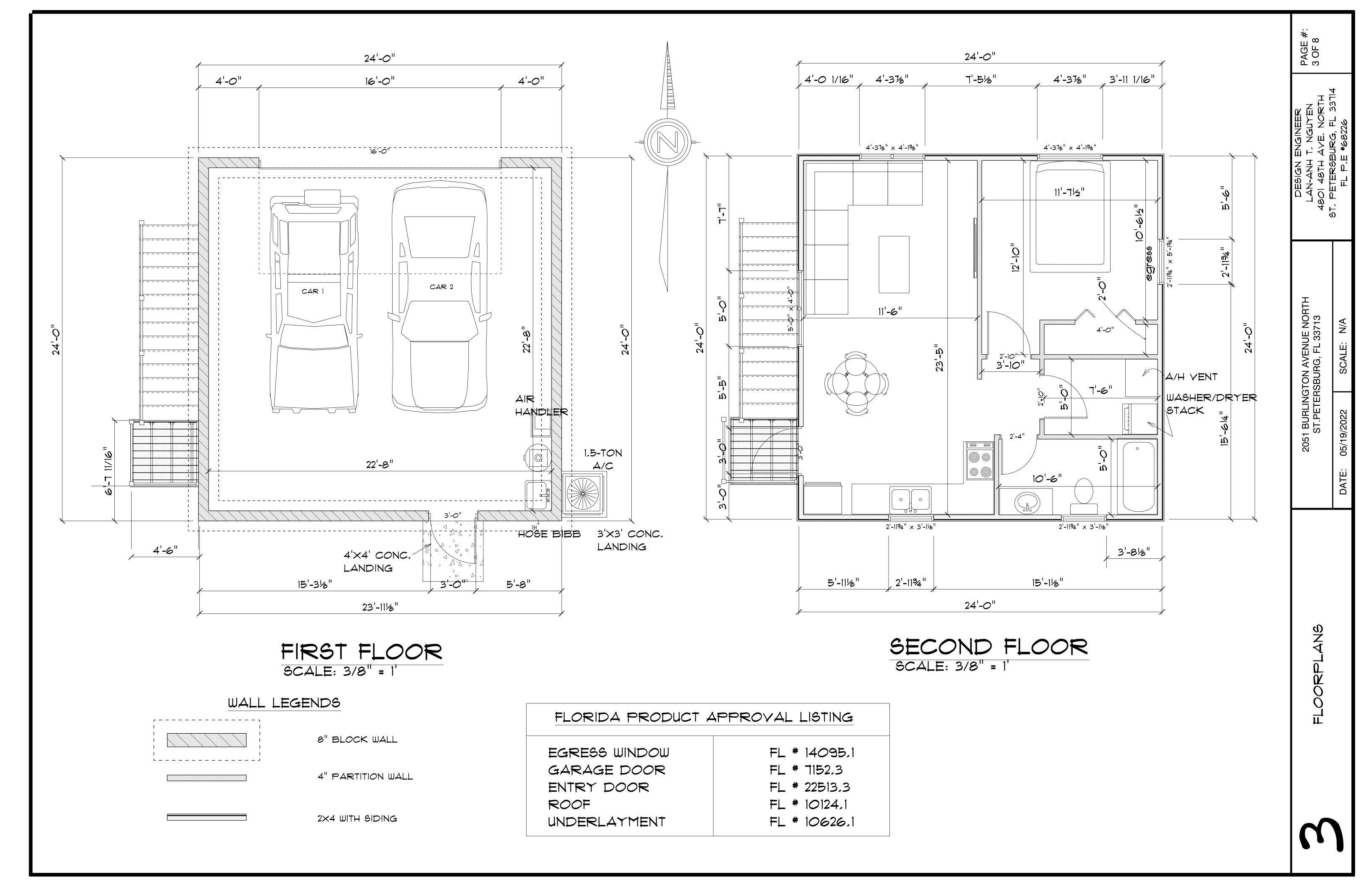
SHEET 8 DETAILS CONT.

SCOPE:

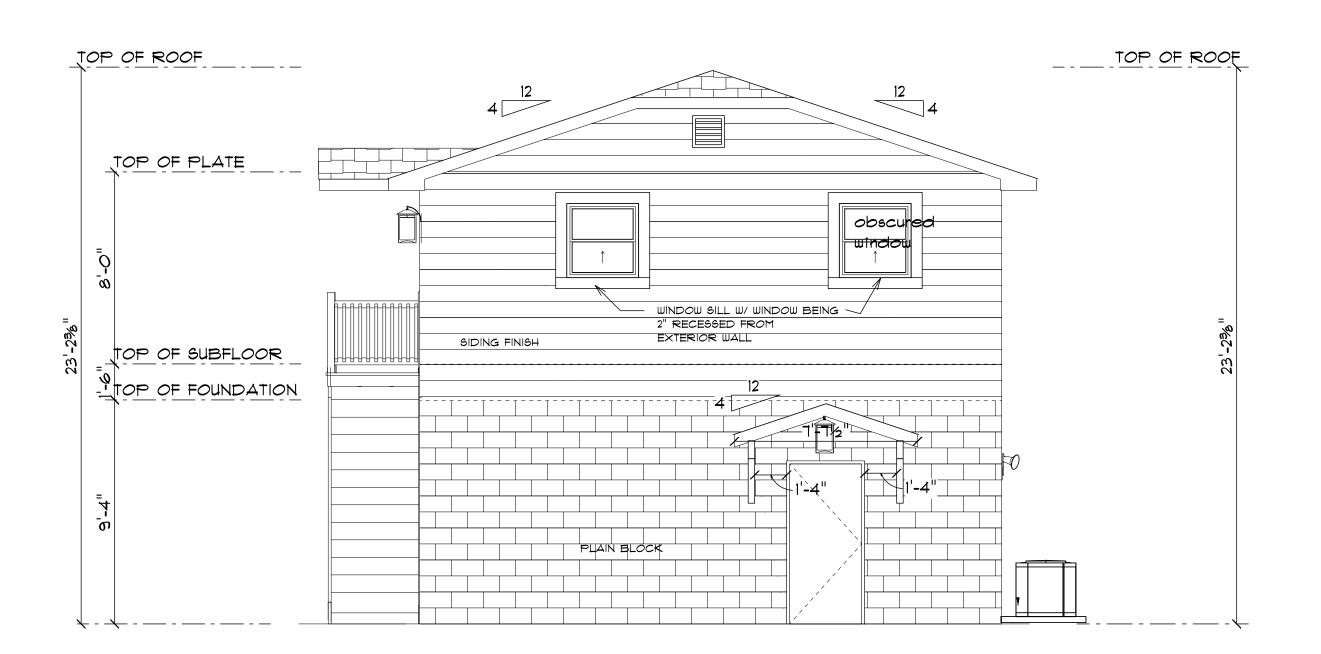
NEW 2-STORY ACCESSORY DWELLING UNIT

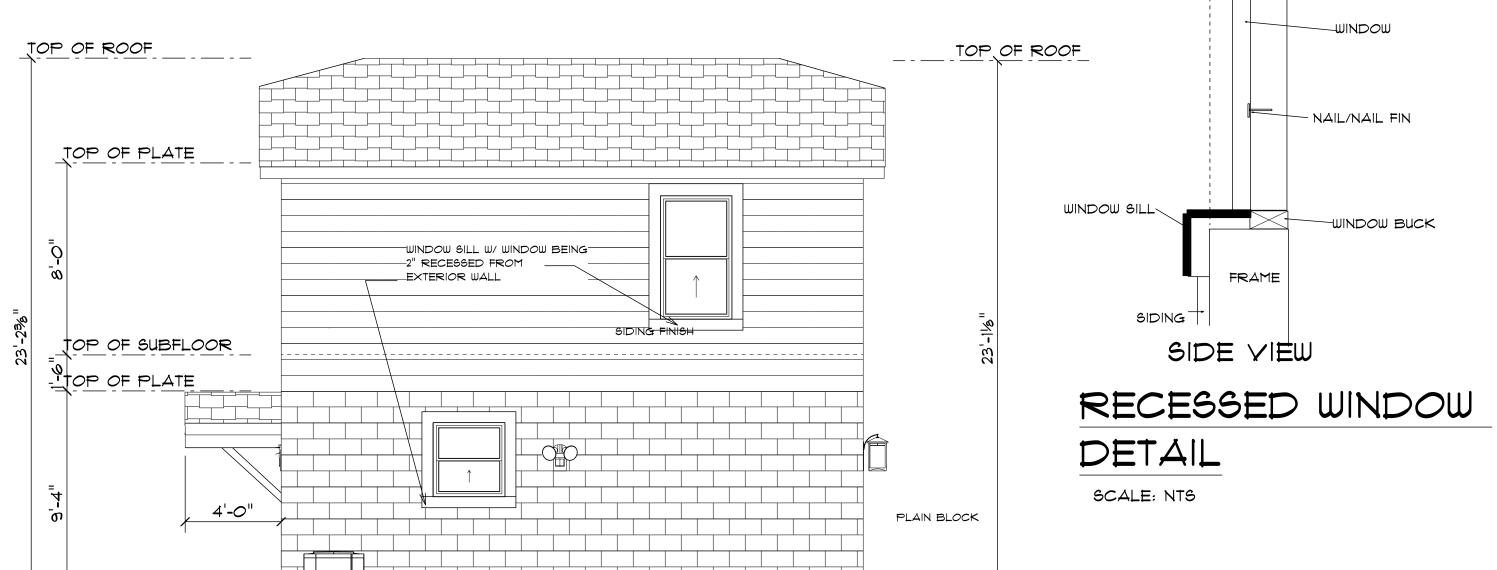






\* PER THE HISTORICAL PRESERVATION COMMITTEE, THE EXPOSED BLOCK IS COMPATIBLE WITH THE CONTRIBUTING RESOURCES, HOWEVER, IF NECESSARY HORIZONTAL SIDING CAN BE ADDED ON THE FIRST FLOOR IF NECESSARY.



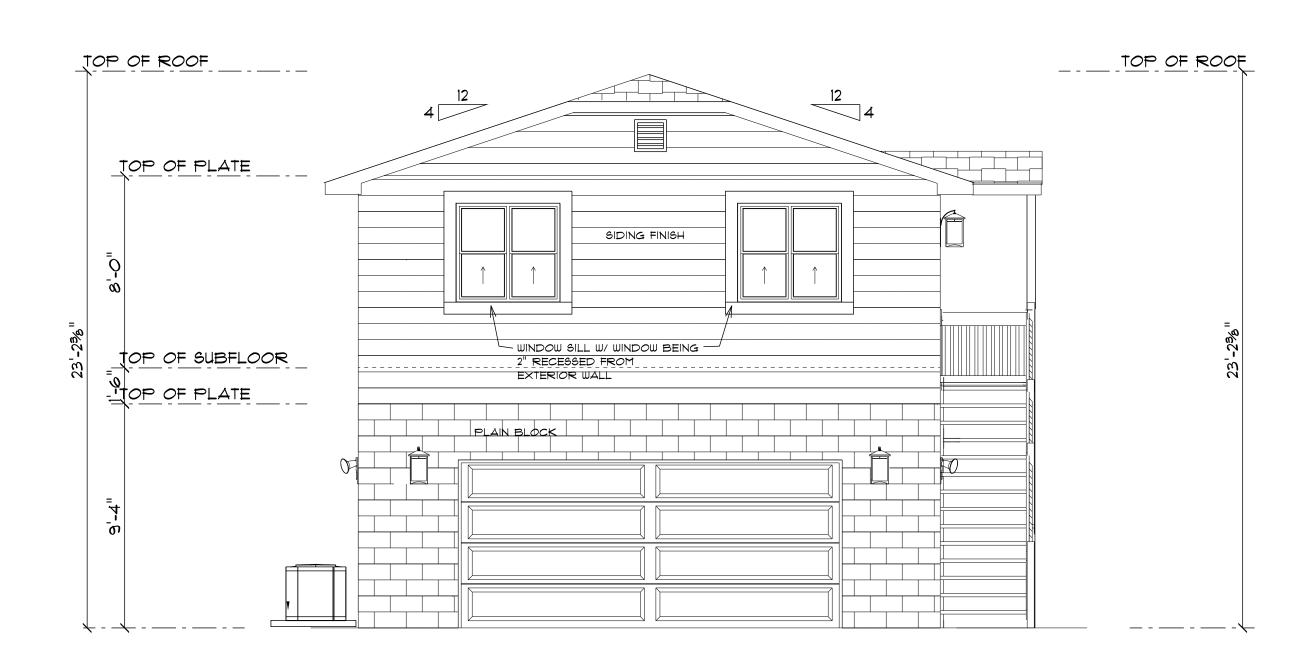


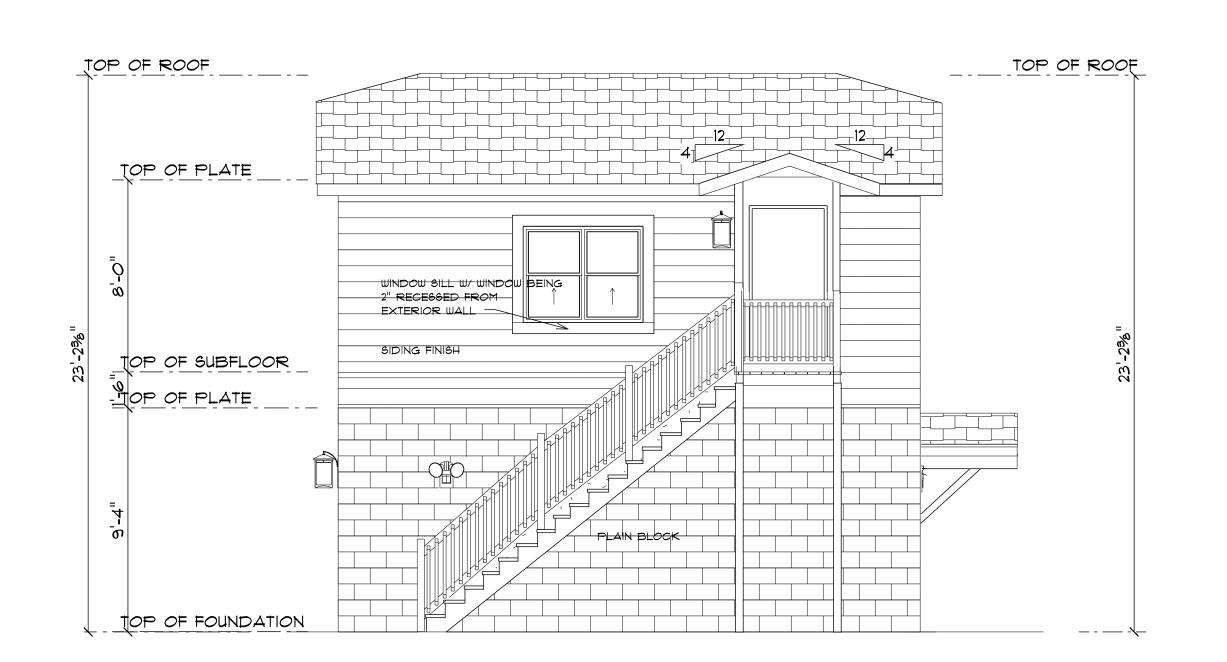
SOUTH ELEVATION

SCALE: 1/4" = 1'

EAST ELEVATION

SCALE: 1/4" = 1'





NORTH ELEVATION

SCALE: 1/4" = 1'

WEST ELEVATION

SCALE: 1/4" = 1'

\* WINDOW TRIM # STYLE TO MATCH THE MAIN HOUSE

4

2051 BURLINGTON AVENUE NORTH ST.PETERSBURG, FL 33713

DATE: 05/19/2022 SCALE: N/A

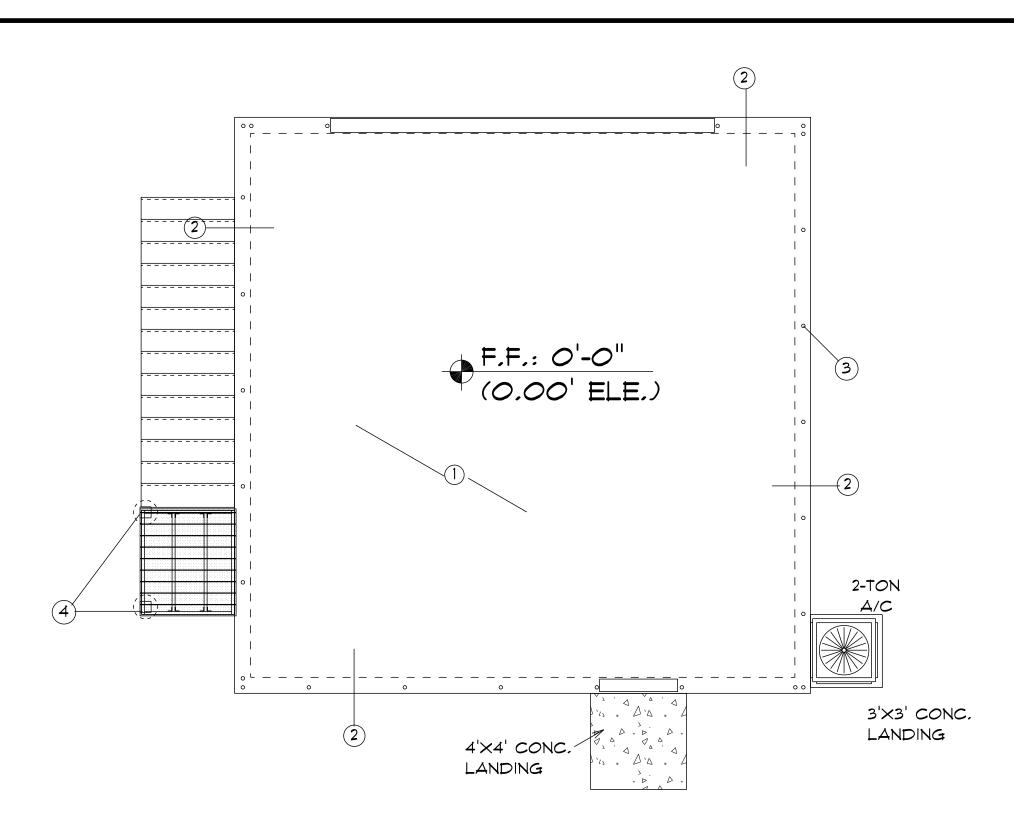
PAGE 4 OF 8

LEYATIONS

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2ND FLOOR- ROOF FRAMING

SCALE: 1/4" = 1'



### IST FLOOR- FLOOR FOUNDATION

SCALE: 1/4" = 1

### TRUSS NOTE

1. ALL PLYWOOD SHEATHING TO BE NAILED 4" O.C. ENDS AND 8" O.C. FIELD UNLESS NOTED

2. CONTACT DESIGN PROFESSIONAL/ENGINEER WITH ANY DISCREPANCIES ON THE PLANS DISCOVERED BY THE TRUSS MANUFACTURE, GENERAL CONTRACTOR AND/OR OWNER.

- 3. HTS16 OR EQUAL AT EACH ROOF TRUSS (FL #13872.3)- UPLIFT 1415LBS TYPICAL TRUSS UPLIFT IS 1320 LBS
- 4, 16" OVERHANG \$ 4:12 ROOF PITCH
- 5, DIMENSIONAL FIBERGLASS SHINGLES (FL\*10124,1) ON TYPE 1 UNDERLAYMENT MOISTURE BARRIER (FL \* 10626.1) ON 7/16" O.S.B. PLYWOOD (TYP.)

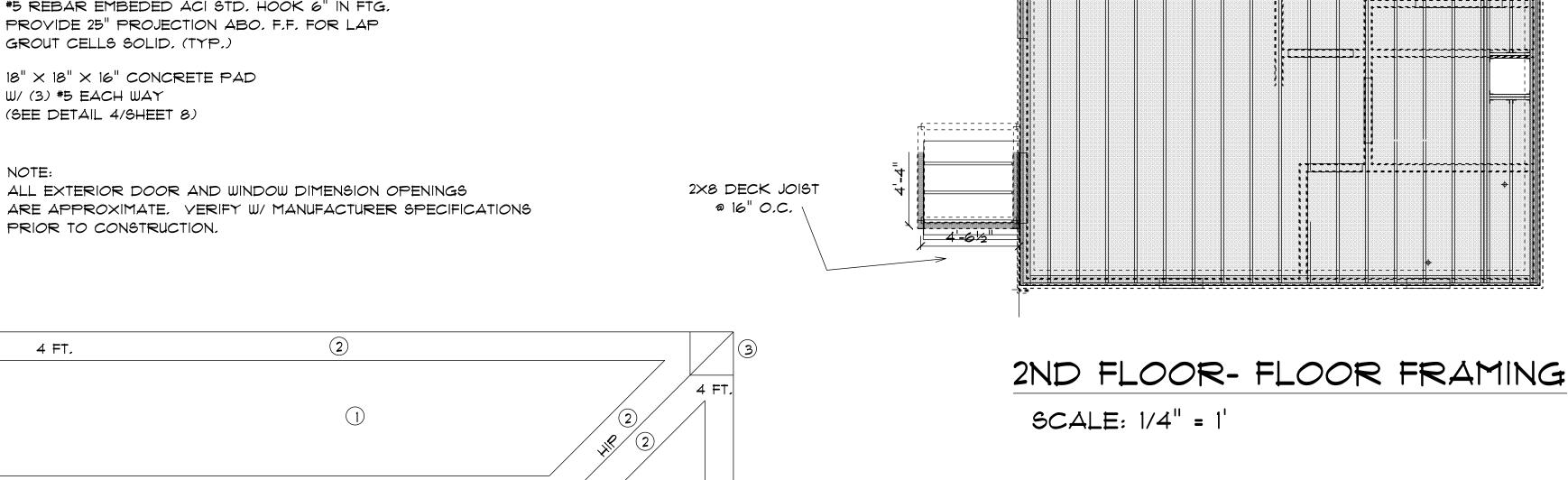
\*\*\*THIS IS A GENERIC LAYOUT TRUSS MANUFACTURER TO PROVIDE SIGNED AND SEALED TRUSS DRAWING INCLUDING BRACING AND STRAPPING DETAILS

### FOUNDATION NOTES

4" CONC SLAB MIN, W/COMPRESSIVE STRENGTH W/6X6/10/10 WW.M OR FIBERMESH REINFORCED TREATED FOR TERMITES, CUT I" DEEP CONTROL JOINTS EVERY 25'-O" EACH WAY AS REQUIRED

- $18' \times 12''$  CONC FOOTER W/ (3) #5 CONT. (SEE DETAIL 2/SHEET 8)
- \*5 REBAR EMBEDED ACI STD, HOOK 6" IN FTG. PROVIDE 25" PROJECTION ABO. F.F. FOR LAP
- 18" × 18" × 16" CONCRETE PAD

ALL EXTERIOR DOOR AND WINDOW DIMENSION OPENINGS ARE APPROXIMATE. YERIFY W/ MANUFACTURER SPECIFICATIONS



PRE-ENGINEERED

TRUSS @ 24" O.C.

PRE-ENGINEERED

2×4 @ 12" O.C.

TRUSS @ 24" O.C. —

## RIDGE2 5 FT. 4 FT. 5 FT. 4 FT. (2) 4 FT. 4 FT,

### NAILING ZONES

ZONE 1: 8d RING SHANK NAILS @ 6" O.C. ON EDGE & 6" O.C. IN FIELD ZONE 2: 8d RING SHANK NAILS @ 6" O.C. ON EDGE & 6" O.C. IN FIELD ZONE 3: 8d RING SHANK NAILS @ 4" O.C. ON EDGE # 4" O.C. IN FIELD

(NOTE: IF 5/8" PLYWOOD IS USED, CONTRACTOR MUST USE IOD RING SHANK NAILS)

NOTE: ROOF SHEATHING - (APA RATED EXPOSURE 1) 1/2" PLYWOOD (TILE) OR 7/16" OSB MIN, (SHINGLES) SPAN RATED 24/16,

SHEATHING TO BE INSTALLED VERTICALLY OR HORIZONTALLY, ATTACHED PER NAILING SCHEDULE PANEL EDGES WILL NEED TO BE ATTACHED TO STUD AND/OR BLOCKING AT ALL EDGES.

## ROOF DIAPHPRAM NAILING SCHEDULE

1. ALL CONNECTIONS SPECIFIED ARE PER SIMPSON CATALOG. CONTACT THE PROFESSIONAL DESIGNER FOR CONNECTOR SUBSTITUTIONS. 2. PRE-ENGINEERED TRUSSES TO BE SPACED @ 24" O.C. UNLESS OTHERWISE NOTED.

3. IF THE TRUSS INFORMATION PROVIDED IN THESE CONSTRUCTION DOCUMENTS IS REVISED BY ANY OTHER PARTY OTHER THAN

PACKAGE REVISIONS DEVELOPED BY OTHERS. 4. ALL TRUSSES TO BE DESIGNED PER ACCEPTABLE PROVISIONS OF THE FLORIDA BUILDING CODE AND APPLICABLE TIMBER CODES: TOP CHORD LL.....20 pef

THE DESIGN ENGINEER FOR REVIEW, THE CLIENT IS RESPONSIBLE FOR ANY CONSTRUCTION COST RESULTING FROM TRUSS

TOP CHORD DL.....10 pef

BOTTOM CHORD LL..... 5 psf

BOTTOM CHORD DL...10 psf

TRUSSES TO BE #2 SOUTHERN PINE OR BETTER

...SEE DESIGN LOADING ON THIS SHEET 5. PRE-ENGINEERED WOOD TRUSS ERECTOR IS REQUIRED TO HANDLE AND INSTALL TRUSSES PER MANUFACTURE'S INSTRUCTION, AS A MINIMUM, INSTALL BRACING IN ACCORDANCE WITH BCSI-I-03, AND LEAVE PERMANENTLY IN PLACE MOVE TO OPPOSITE SIDE OF CHORD IF NECESSARY DUE TO INTERFERENCE WITH SHEATHING OR CEILING MATERIALS, TOP AND BOTTOM CHORD MEMBERS OF THE PRE-ENGINEERED TRUSS SYSTEM FOR THE COMPLETE STRUCTURE ARE TO BE CONTINUOUSLY BRACED BY SHEATHING AND CEILING FINISHES, THE ROOF TRUSS SYSTEM IS NOT COMPLETED UNTIL ALL BRACING, SHEATHING AND FINISHES ARE PERMANENTLY ATTACHED.

### FRAMING NOTES:

THE ARCHITECT, DESIGNER AND/OR ENGINEER OF RECORD HAS NOT REVIEWED THE PRE-ENGINEERED WOOD TRUSS LAYOUT OR ENGINEERING TO VERIFY LOAD PATHS, THE ARCHITECT, DESIGNER AND/OR ENGINEER RESERVES THE RIGHT TO MAKE ANY

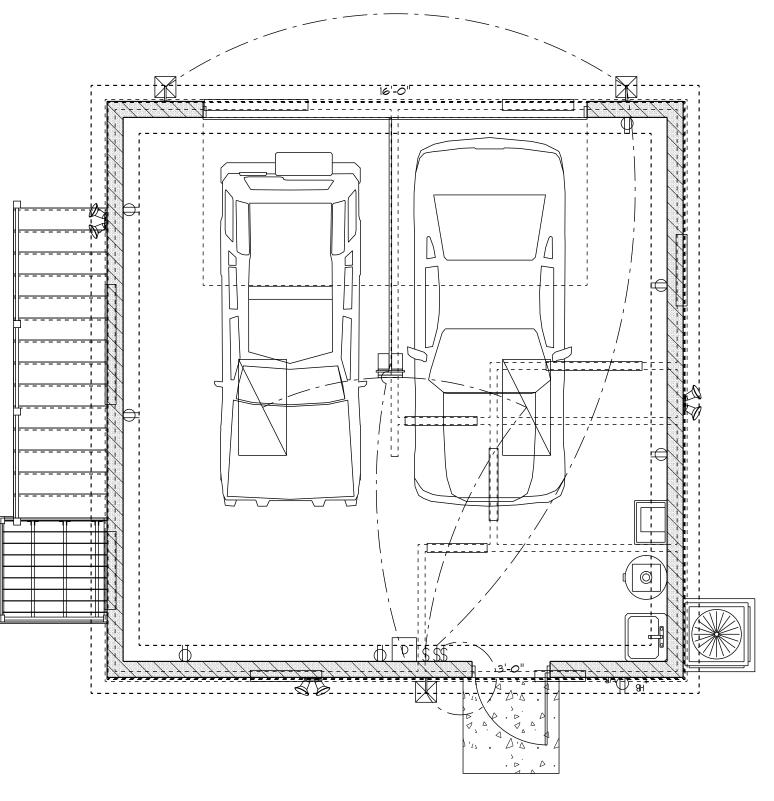
A LICENSED CERTIFIED CONTRACTOR SHALL BE ON SITE AT THE TIME THE TRUSSES ARE BEING ERECTED.

ALL ANCHORS, STRAPS, CLIPS AND CONNECTORS TO BE INSTALLED PER SIMPSON STRONG-TIE OR EQUAL SPECIFICATIONS.

PERMANENT LATERAL BRACES AS SPECIFIED BY TRUSS DESIGNER SHALL BE DIAGONALLY RESTRAINED TO PREVENT BUCKLING WITH 45° DEG, "X" BRACING AT 20'-0" O.C. OR TERMINATION WHICH EVER IS LESS, "X" BRACING SHALL BE 2×4 #2 S.Y.P. OR BETTER,

REVISIONS DEEMED NECESSARY AFTER THE TRUSS ENGINEERING AND LAYOUT IS SUPPLIED.

TRUSS MANUFACTURE TO YERIFY ALL TRUSS & LYL BEAM SIZING NOTED.



### GARAGE/FIRST FLOOR ELECTRICAL

SCALE: 1/4"= 1"

ELECTRICAL LEGEND					
ELECTRICAL	COUNT	SYMBOL			
fluorescent light 2 x 4	2				
exterior craftsman light fixture	3	X			
spotlight double with motion detector	3				
garage door opener with track	1				
garage door opener	1	D			
outlet	7	Ф			
outlet wp	1	<b>₩</b> P			
switch	1	\$			
switch double	1	\$\$			

### ELECTRICAL NOTES:

1. ALL WORK SHALE BE IN STRICT ACCORDANCE WITH NATIONAL ELECTRICAL CODE ALL APPLIANCE LOCAL ORDINANCES AND IN COOPERATION WITH THE UTILITY COMPANIES.

COOPERATION WITH THE UTILITY COMPANIES.

2. COORDINATE ALL WORK WITH TRADES.

PRIOR TO INSTALLATION.

3. MOUNT ALL OUTLETS AND RECEPTACLES PLUMB AND FLUSH WITH WALLS.
4. RECEPTACLES SHALL BE MOUNTED 12" A.F.F. UNLESS OTHERWISE NOTED.
5. INSTALL ELECTRICAL CABLE, WIRE AND CONNECTORS AS INDICATED IN ACCORDANCE WITH THE MANUFACTURE'S WRITTEN INSTRUCTION, THE APPLIANCE REQUIREMENTS OF NEC AND THE NATIONAL ELECTRICAL CONTRACTORS ASSOCIATION "STANDARD OF INSTALLATION" AND IN

ACCORDANCE WITH RECOGNIZED INDUSTRY PRACTICES TO ENSURE THAT PRODUCTION SERVE THE INTENDED FUNCTIONS.

6. ALL WIRE SHALL BE STD. #10 AND #12 AND SHALL BE ROMEX OR NM CABLE TYPE B OR APPROVED EQUAL.

1. COORDINATE ELECTRICAL REQUIREMENTS FOR A/C WITH A/C CONTRACTOR. 8. ALL RECEPTACLES IN WET AREAS SHALL BE ON AN GFCI (GROUND

FAULT CIRCUIT INTERRUPTER) PROTECTED CIRCUIT,

9. ALL RECEPTACLES SHALL BE ON AN AFCI

(ARC-FAULT CIRCUIT INTERRUPTER) PROTECTED CIRCUIT,
10. YERIFY ALL OUTLETS AND SWITCHES WITH OWNER AND/OR G.C.

11. ALL SMOKE DETECTORS HARWIRED IN SERIES WITH BATTERY BACKUP.

SCALE	:: 1/4"= 1'		
	ELECTRICA	AL LEGEN	ID
	ELECTRICAL	COUNT	SYMBOL
	ceiling fan spotlights 01	2	
	can light 6inch	8	0
	exterior craftsman light fixture	1	
	fan	1	₩
	outlet	15	Ф
	outlet 220v	1	•
	outlet gfi	3	∯ <sup>GFI</sup>
	smoke detector	3	•
	switch	5	\$
	switch double	2	\$\$
	switch triple	1	\$\$\$

vanity bar light 02

1 000

SECOND FLOOR ELECTRICAL

4'-3%" × 4'-1%"

2'-11¾" × 4'-1%"

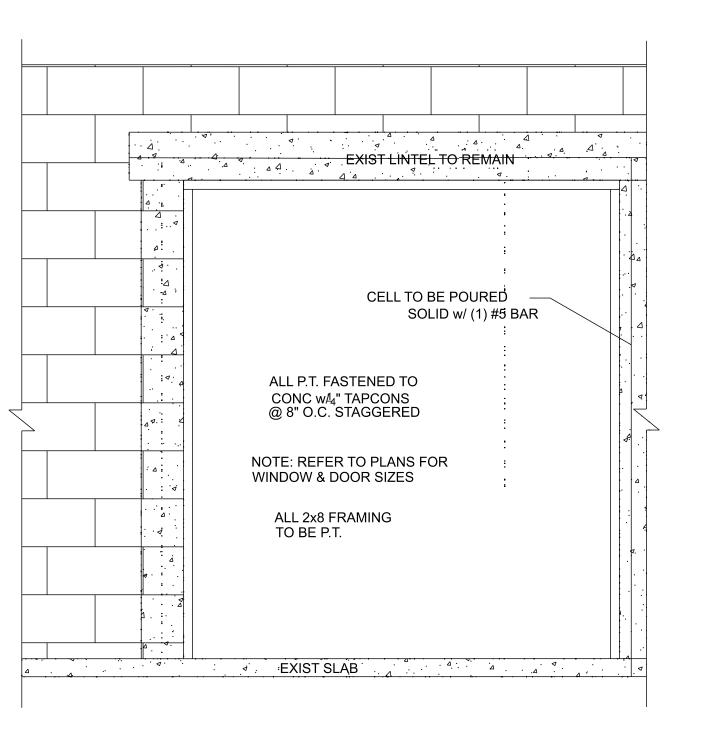
### WIND-BORNE DEBRIS PROTECTION FASTENNING SCHEDULE FOR WOOD STRUCTURE PANELS

DESIGN WIND LOAD: 145MPH (3 SEC. GUSTS) EXPOSURE CATEGORY E

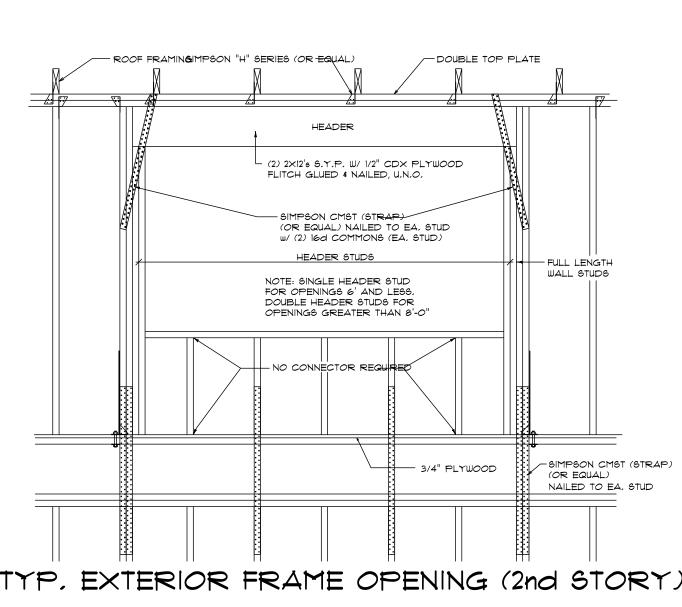
	FASTENER SPACING (INCHES) *,**			
FASTENER TYPE	PANEL SPAN 2 FEET	2 FEET < PANEL SPAN < 4 FEET	4 FEET ( PANEL SPAN (6 FEET	6 FEET ( PANEL SPAN (8 FEET
2 1/2 #6 WOOD SCREWS***	16	16	12	9
2 1/2 #8 WOOD SCREWS***	16	16	16	12
DOUBLE-HEADED NAILS****	12	6	4	3

- st This table is base on a max, wind speed of 145 mph and mean roof height of 33' or less,
- \*\* Fasteners shall be installed at opposing ends of the wood structural panel.
- \*\*\* Where srews are attatch to masonry or masonry/ stuuco, they shall be attached using vibration-registant anchors having a minimum withdrawal capacity of 490 lb.
- \*\* Fasteners shall be installed at opposing ends of the wood structural panel.

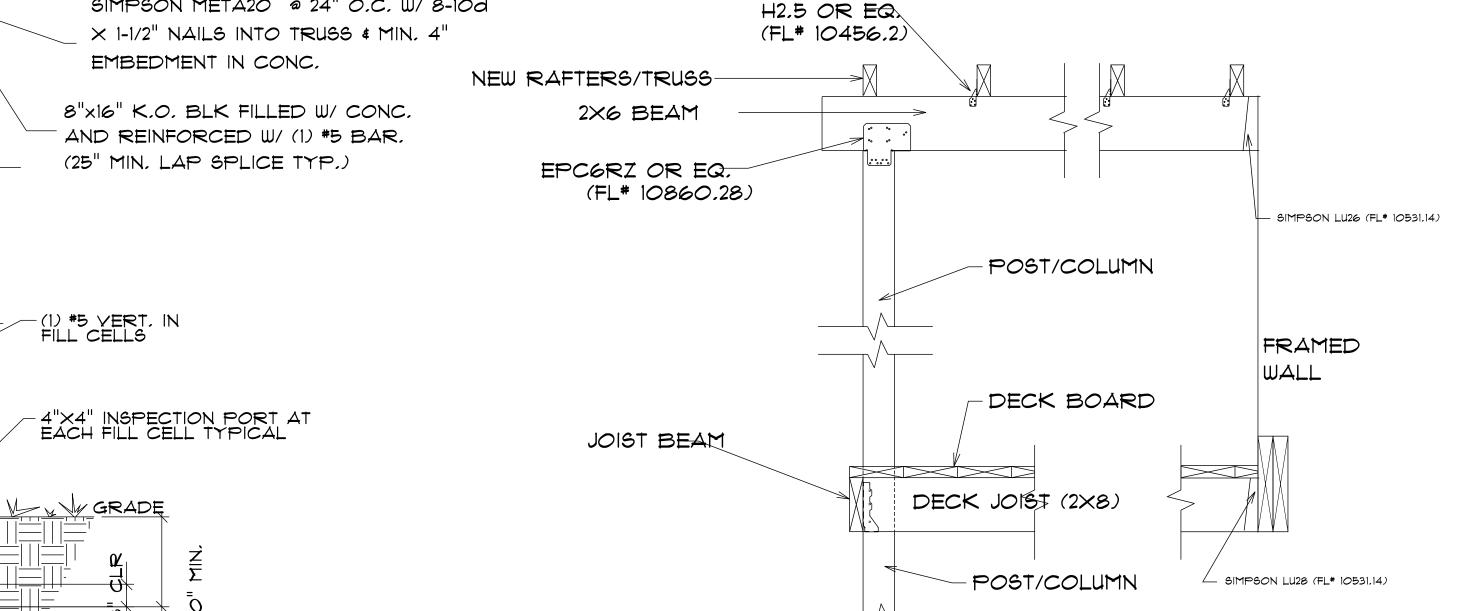
Note: Permanently installed hardware shall be non-corrosive

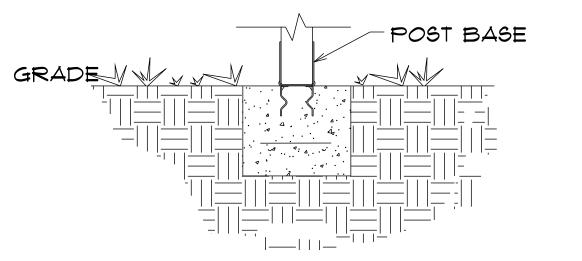


DOOR OPENING REINFORCEMENT DETAIL









POST/DECKING DETAIL

WALL SECTION DETAIL-2 STORY

NTS

12

1/2" DENSE CEILING BOARD DOUBLE 2x4 TOP PLATE

RI3 BATT INSULATION (TYP.)

5/8" TYP X. GYPSUM BRD.

1×2 P.T. FURRING

STD. 90° HOOK W/ 4"

#5 DOWEL TO MATCH VERTICAL REINFORCING W/25" MINIMUM LAP

4" THICK CONC. SLAB

WITH 6x6-10/10 W.W.M.

ON 6 MIL VAPOR

BARRIER

CLEAN, POISONED AND COMPACTED

EMBED INTO K.O. BLK

STRIP @ 24" 0/C.

2x4 SPF STUDS @ 16" 0/C-

R38 BATT INS,-

3/4" PLYWOOD DECKING GLUED

AND NAILED W/ 8d COMMON @

6" O/C EDGES AND 12" O/C FIELD

VARIES

SIMPSON SP2 @ EA. STUD

2x4 MID-POINT BLOCKING

1/2" DRYWALL

2×4 BOT, PLATE

PRE-ENGINEERED

FLOOR TRUSSES.

PRE ENGINEERED WD TRUSS @ 24" O.C. MAX.

"ELK PRESTIQUE PLUS" SHINGLES

APA RATED 7/16" OSB SHEATHING WITH EXPOSURE 1 DESIGNATION

ALUM, DRIP EDGE SECURED

MOISTURE BARRIER ON 5/8" PLYWOOD

SHEATHING (COLOR & STYLE TO MATCH EXISTING)

SIMPSON CS20 STRAP @ 16" O/C W/ (5) 8d NAILS

INTO STUD \$ (5) 8d NAILS INTO BAND JOIST

W/ 6d NAILS @ 12" O/C.

ALUM, OR WOOD FASCIA

ALUM, OR P.W. YENTED SOFFIT

7/16" OSB SHEATHING NAILED

W/8d NAILS @ 6" O/C EDGES

SIMPSON META20 @ 24" O.C. W/ 8-10d

YINYL SIDING FINISH ON

AND 12" O/C FIELD,

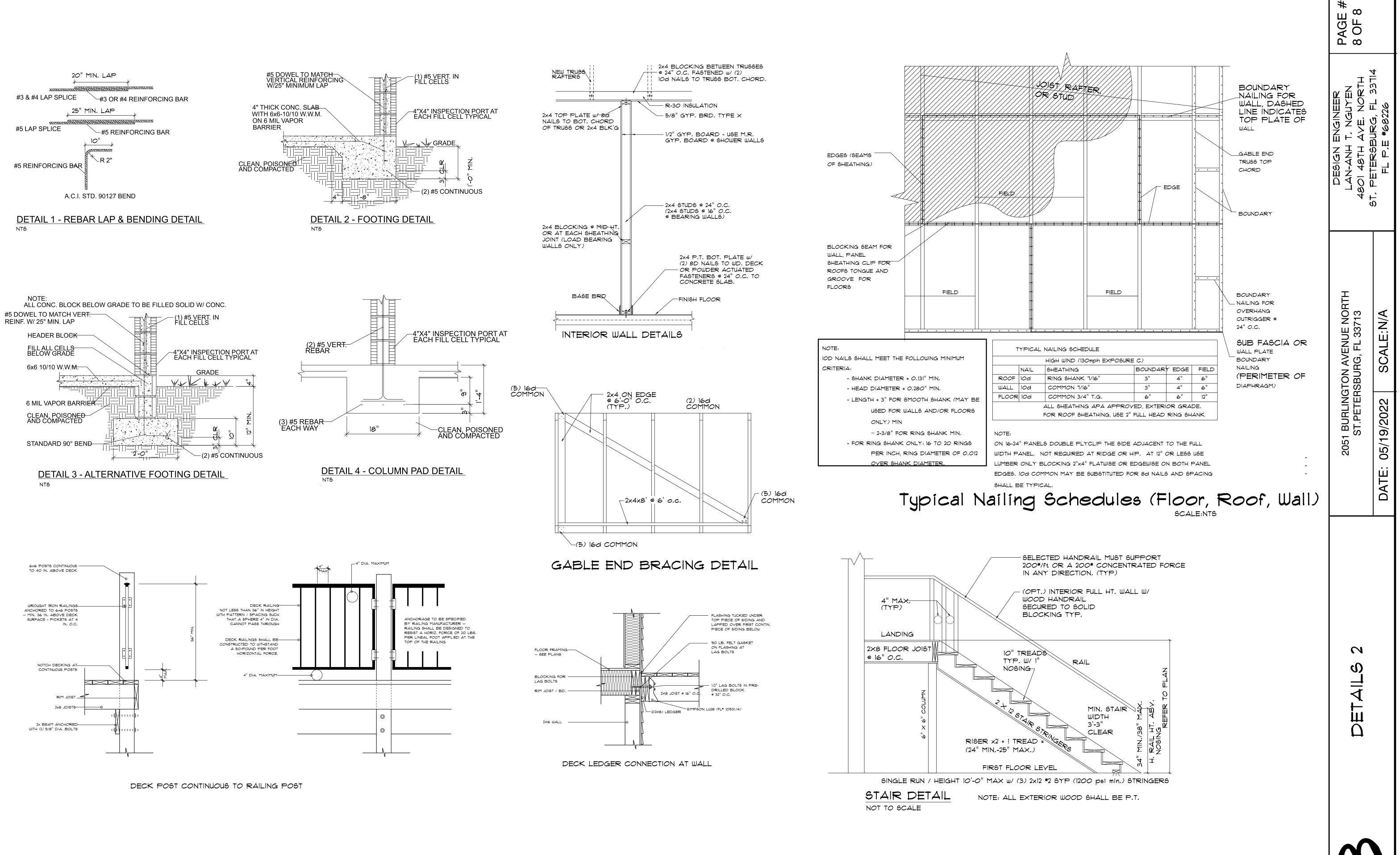
2x8 BAND JOIST

METAL SEAT PLATE

(3) #5 CONTINUOUS

SIMP, MTS16 STRAP @ EACH TRUSS

W/ (6) NAILS PER SHINGLE, ROOFING UNDERLAYMENT



**WOOD-EDGE STEEL DOOR UNIT** 8'-0" DOUBLE DOOR WITH / WITHOUT SIDELITES

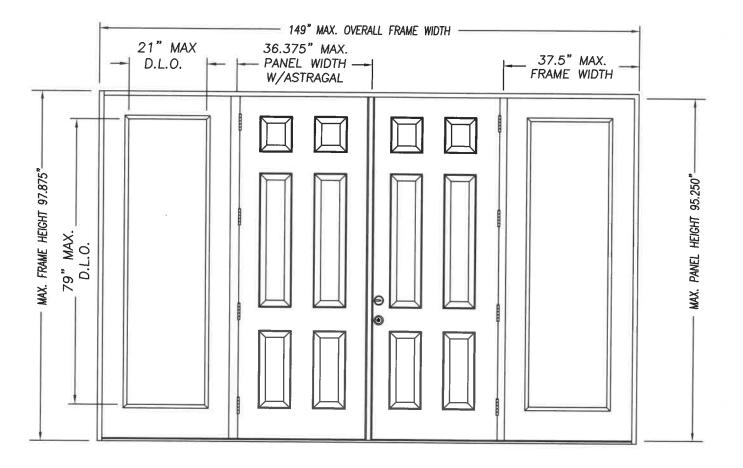
### GENERAL NOTES

- 1. EVALUATED FOR USE IN LOCATIONS ADHERING TO THE FLORIDA BUILDING CODE AND WHERE PRESSURE REQUIREMENTS AS DETERMINED BY ASCE 7, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, DOES NOT EXCEED THE DESIGN PRESSURES LISTED.
- 2. WHEN INSTALLED IN THE WIND-BORNE DEBRIS REGION OR THE HIGH VELOCITY HURRICANE ZONE (HVHZ). HURRICANE PROTECTIVE SYSTEM IS NOT REQUIRED ON OPAQUE PANELS, BUT IS REQUIRED ON SIDELITES.
- 3. POLYURETHANE CORE FLAME SPREAD INDEX OF 50 AND SMOKE DEVELOPED INDEX OF 60 PER ASTM E84.
- 4. PLASTICS TESTING:

TEST DESCRIPTION	DESIGNATION	LITE FRAME
SELF IGNITION TEMP	ASTM D1929	740 °F > 650 °F
RATE OF BURNING	ASTM D635	0.77 IN/MIN
SMOKE DENSITY	ASTM D2843	13.4%
TENSILE STRENGTH*	ASTM D638	7.1% DIFF

\* COMPARATIVE TENSILE STRENGTH AFTER WEATHERING 4500 HOURS XENON ARC METHOD 1

Addendum to NAM Certification No.: NIO/3747 Reviewed By: Date Reviewed: 7/27/20



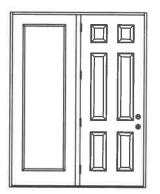
DOUBLE DOOR UNIT W/SIDELITES



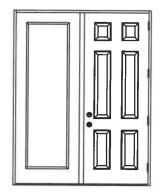




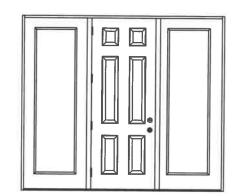
DOUBLE DOOR UNIT



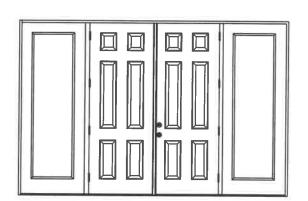
SINGLE DOOR UNIT WITH SIDELITE



SINGLE DOOR UNIT WITH SIDELITE



SINGLE DOOR UNIT W/SIDELITES



DOUBLE DOOR UNIT W/SIDELITES

	TABLE OF CONTENTS		
SHEET #	DESCRIPTION		
1	TYPICAL ELEVATIONS & GENERAL NOTES		
2	ANCHORING LOCATIONS & DETAILS		
3	ANCHORING LOCATIONS & DETAILS		

BEOLOGI BESSELVES STREET						WHERE WATER INFILTRATION PERFORMANCE IS					
				SSURE RA	TING	REQUIRED TO BE 15% OF DESIGN PRESSURE					
CONFIG	MAX WIDTH	INS	WING	OUTS	WING	INSWING	BUMPER O/S	Z-SERIES O/S	HIGH DAM O/S		
X	37.5"	+70.0	-70.0	+70.0 /	-70.0	N/A	N/A	N/A	+70.0 / -70.0		
XX	74"	+45.0 /	-50.0	+50.0	-45.0	N/A	N/A	N/A	N/A		
OX or XO	75"	+45.0 /	-50.0	+50.0	-45.0	N/A	N/A	N/A	N/A		
OXO	112.5"	+45.0	-50.0	+50.0	-45.0	N/A	N/A	N/A	N/A		
OXXO	149"	+45.0	-50.0	+50.0	-45.0	N/A	N/A	N/A	N/A		

DWG. BY:

CHK. BY: DRAWING NO .:

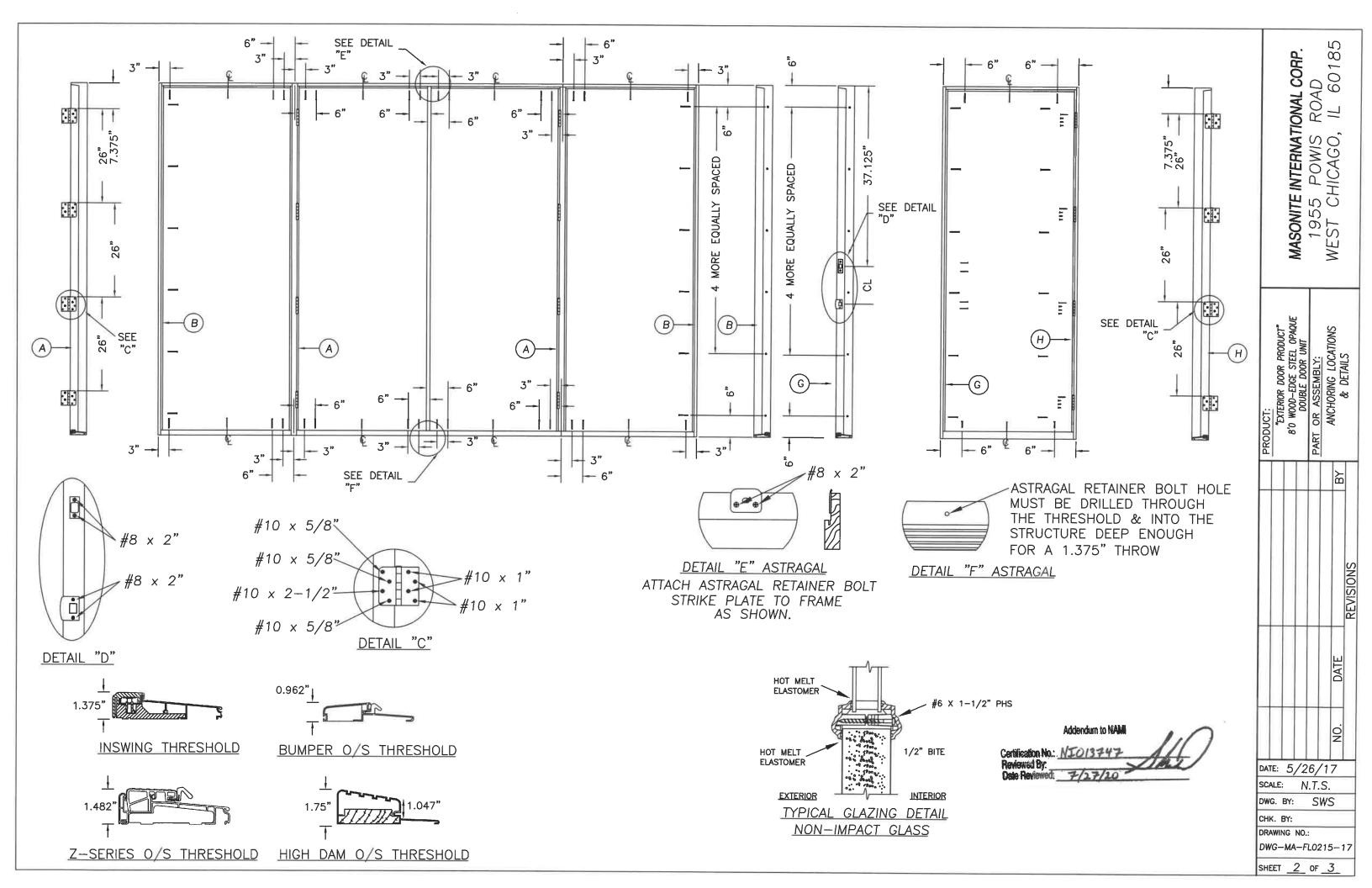
SWS

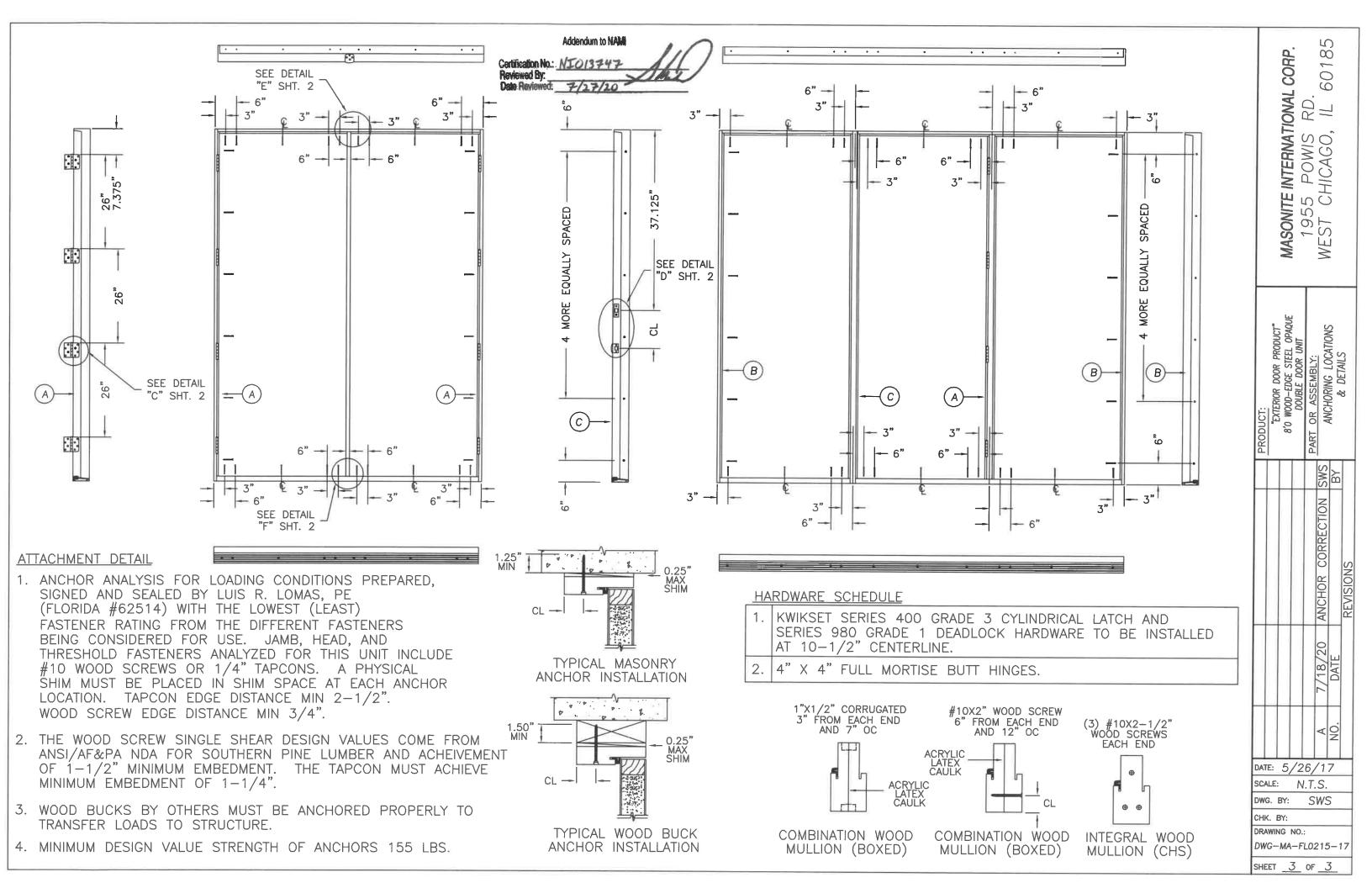
DWG-MA-FL0215-17

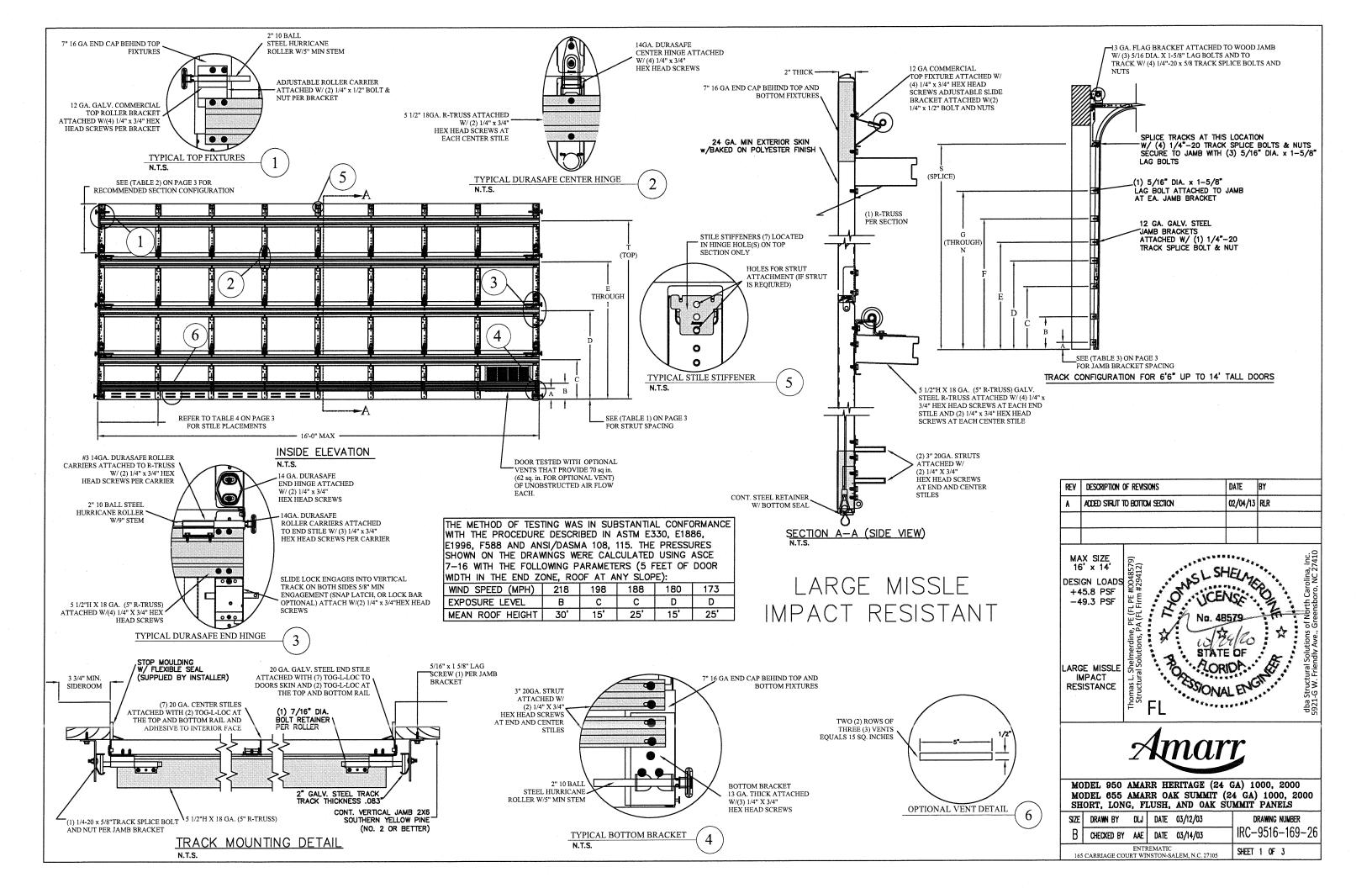
SHEET 1 OF 3

KURT BALTHAZOR FLORIDA P.E. #56533

PRODUCT:  "EXTERIOR DOOR PRODUCT"  DOUBLE 8'0" OPAQUE WOOD-EDGE STEEL DOOR WOOD-EDGE STEEL DOOR PART OR ASSEMBLY: TYPICAL ELENATIONS & GENERAL NOTES
7/18/20 MISC EDITS DATE REVISIONS
∀ NON
DATE: 5/26/17  SCALE: N.T.S.

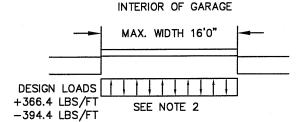






### SPECIFICATIONS AND NOTES

- 1. ALL THE LOAD FROM THE DOOR IS TRANSFERRED TO THE VERTICAL TRACK, FROM THE TRACK THE LOAD IS TRANSFERRED TO THE VERTICAL JAMBS. THE HORIZONTAL JAMB OR HEADER RECEIVES NO PORTION OF THE LOAD TRANSFERRED FROM THE DOOR.
- 2. EACH VERTICAL JAMBS RECEIVES MAXIMUM DESIGN LOADS OF: +366.4 LBS/FT & -394.4 LBS/FT
- 3. DOOR AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
- 4. DOOR SECTIONS SHALL BE 24 GA. (.0216) MIN. EXTERIOR SKIN ROLLED FORMED, W/ BAKED ON POLYESTER FINISH
- 5. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
- USE (1) 5 1/2" R-TRUSS PER SECTION & (2) 3" 20GA STRUTS AT BOTTOM SECTION
- 6. DOORS OVER (4) SECTIONS REFER TO TABLES 1 AND 2 ON PAGE 3
- 7. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTRED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.



AVAILABLE TRACK CONFIGURATIONS

N.T.S.

### WOOD JAMB ATTACHMENT TO STRUCTURE

2 X 6 VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE 5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 12" O.C. (1 1/2" EMBEDMENT)

2 X 6 VERTICAL JAMB ATTACHMENT TO 2,000 PSI CONCRETE

FASTENERS FOR HOLLOW C-90 BLOCK)

HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C. (2 1/2" EMBEDMENT)

HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 12" O.C. (1 1/4" EMBEDMENT) ITW/RAMSET REDHEAD (TRU-BOLT) 3/8" X 4" STARTING 6" FROM ENDS THEN 22" O.C. (2 1/2" EMBEDMENT)

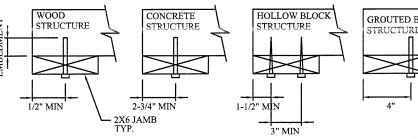
X 6 VERTICAL JAMB ATTACHMENT TO HOLLOW C-90 BLOCK SIMPSON 1/4" X 3" TITEN SCREWS STARTING 6" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 8" O.C.

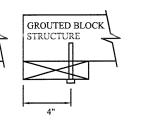
(1 1/2" EMBEDMENT) HILTI 1/4" X 2-3/4" KWIK-CON II+ SCREWS STARTING 6" FROM ENDS, USE PAIRS OF FASTENERS (3" APART)

AT 8" O.C. (1 1/4" EMBEDMENT)  $\frac{2 \text{ X 6 VERTICAL JAMB ATTACHMENT TO GROUTED C-90 BLOCK (2000 PSI GROUT)}}{\text{HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 14" O.C. (1 1/4" EMBEDMENT) (OR, USE 1 1/4" 1/4" EMBEDMENT) (OR, USE 1$ 

\*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE. \*PREPARATION OF WOOD JAMBS BY OTHERS

WOOD CONCRETE STRUCTURE STRUCTURE





MAX SIZE 16' x 14' DESIGN LOADS +45.8 PSF -49.3 PSF	PE (FL PE #0048579) \( (FL Firm #29412)	, V.	SL SHELM CENSON	Sola	of North Carolina, Inc.
	elmerdine, Solutions, P/	3	STATE OF	o 🌣	Solutions o

DATE 02/04/13 RLR

LARGE MISSLE RESISTANCE

REV DESCRIPTION OF REVISIONS

ADDED STRUT TO BOTTOM SECTION

MODEL 950 AMARR HERITAGE (24 GA) 1000, 2000 MODEL 655 AMARR OAK SUMMIT (24 GA) 1000, 2000 SHORT, LONG, FLUSH, AND OAK SUMMIT PANELS

I	SIZE	SIZE DRAWN BY		DATE	03/12/03	DRAWING NUMBER
	В	CHECKED BY	AAE	DATE	03/14/03	IRC-9516-169-26
	165	CARRIAGE COU	SHEET 2 OF 3			

### TABLE 1

DOOR		STRUT	SPACI	VG (BAS	SED ON	RECOM	MENDED	SECTIO	V	
HEIGHT		-		CC	NFIGUE	RATION)				TOP
	Α	В	С	D	E	F	G	Н	I	T.
6' 6"	4 1/2"	7"	17 1/2"	35 1/2"	53 1/2"					70 1/2"
7'	4 1/2"	7"	17 1/2"	38 1/2"	59 1/2"					76 1/2"
7' 6"	4 1/2"	7"	14 1/2"	32 1/2"	50 1/2"	68 1/2"				82 1/2"
8'	4 1/2"	7"	17 1/2"	35 1/2"	53 1/2"	71 1/2"				88 1/2"
8' 6"	4 1/2"	7"	17 1/2"	38 1/2"	59 1/2"	77 1/2"				94 1/2"
9'	4 1/2"	7"	14 1/2"	32 1/2"	50 1/2"	68 1/2"	86 1/2"			100 1/2"
9' 6"	4 1/2"	7"	17 1/2"	35 1/2"	53 1/2"	71 1/2"	89 1/2"			106 1/2"
10'	4 1/2"	7"	17 1/2"	38 1/2"	59 1/2"	77 1/2"	95 1/2"			112 1/2"
10' 6"	4 1/2"	7"	17 1/2"	38 1/2"	59 1/2"	80 1/2"	101 1/2"			118 1/2"
11'	4 1/2"	7"	17 1/2"	35 1/2"	53 1/2"	71 1/2"	89 1/2"	107 1/2"		124 1/2"
11' 6"	4 1/2"	7"	17 1/2"	38 1/2"	59 1/2"	77 1/2"	95 1/2"	113 1/2"		130 1/2"
12'	4 1/2"	7"	17 1/2"	38 1/2"	59 1/2"	80 1/2"	101 1/2"	119 1/2"		136 1/2"
12'6"	4 1/2"	7"	17 1/2"	35 1/2"	53 1/2"	71 1/2"	89 1/2"	107 1/2"	125 1/2"	142 1/2"
13'	4 1/2"	7"	17 1/2"	38 1/2"	59 1/2"	77 1/2"	95 1/2"	113 1/2"	131 1/2"	148 1/2"
13' 6"	4 1/2"	7"	17 1/2"	38 1/2"	59 1/2"	80 1/2"	101 1/2"	119 1/2"	137 1/2"	154 1/2"
14'	4 1/2"	7"	17 1/2"	38 1/2"	59 1/2"	80 1/2"	101 1/2"	122 1/2"	143 1/2"	160 1/2"

### TABLE 3

IADLE 3															
DOOR						TRAC	KAT	TACH	MEN	IT					SPLICE
HEIGHT	Α	В	С	D	E	F	G	Н	I	J	K	L	М	N	S
6' 6"	3"	14"	27"	38"	46"	56"	64"								70"
7'	3"	14"	27"	38"	46"	56"	68"							-	76"
7' 6"	3"	14"	27"	38"	46"	56"	68"	78"							82"
8'	3"	14"	27"	38"	46"	56"	68"	78"							88"
8' 6"	3"	14"	27"	38"	46"	56"	68"	78"	88"						94"
9'	3"	14"	27"	38"	46"	56"	68"	78"	88"						100"
9' 6"	3"	14"	27"	38"	46"	56"	68"	78"	88"	98"					106"
10'	3"	14"	27"	38"	46"	56"	68"	78"	88"	100"					112"
10' 6"	3"	14"	27"	38"	46"	56"	68"	78"	88"	100"	110"				118"
11'	3"	14"	27"	38"	46"	56"	68"	78"	88"	100"	110"				124"
11' 6"	3"	14"	27"	38"	46"	56"	68"	78"	88"	100"	110"	120"			130"
12'	3"	14"	27"	38"	46"	56"	68"	78"	88"	100"	110"	122"			136"
12'6"	3"	14"	27"	38"	46"	56"	68"	78"	88"	100"	109"	122"	132"		142"
13'	3"	. 14"	27"	38"	46"	56"	68"	78"	88"	100"	114"	122"	134"		148"
13' 6"	3"	14"	27"	38"	46"	56"	68"	78"	88"	100"	109"	122"	134"	144"	154"
14'	3"	14"	27"	38"	46"	56"	68"	78"	88"	100"	114"	122"	134"	146"	160"

ALL TRACK ATTACHMENT SPACING +/- 2" ALLOWED WITH SYP NO.2 OR BETTER ONLY

### TABLE 4

Section	Daniel Tona				er Stile Loc red from Lo			
Width	Panel Type	1st	2st	3rd	4th	5th	6th	7th
(ft)		(in)	(in)	(in)	(in)	(in)	(in)	(in)
10' 0	Short	25.218	48.406	71.594	94.782			
10' 0	Long	30,000		90.000			<del></del>	
10' 0	Bead	30.625		89.375				
12' 0	Short		48.812	72.000	95.188	118.376		
12' 0	Long		49.625	72.000	94.375	116.750	<del></del>	<del>                                     </del>
12' 0	Bead		48.313	72.000	95.688	119.375		
12' 2	Short		49.636	73,000	96.364	119.728		<del> </del>
12' 2	Long	<del></del>	50.084	73,000	95.916	118.832		<u> </u>
12' 2	Bead	<u> </u>	49.063	73.000	96.938	120.875		
12' 4	Short		50,636	74.000	97.364	120.728		
12' 4	Long		51.084	74.000	96.916	119.832		
12' 4	Bead		49.813	74.000	98,188	122.375		
12' 6	Short		50.670	75,000	99,330	123.660		<del> </del>
12' 6	Long		51.170	75.000	98.830	122.660	<del> </del>	<u> </u>
12' 6	Bead		50.563	75.000	99.438	123,875	<b>-</b>	
12' 8	Short		51.670	76.000	100.330	124.660	<del></del>	
12' 8			52.100	76.000	99,900	123,800		ļ
	Long							
12' 8	Bead		51.313	76.000	100.688	125.375		
12' 10	Short		52.250	77.000	101.750	126.500		
12' 10	Long		53,100	77.000	100,900	124.800	<b></b>	ļ
12' 10	Bead	27.125	52.063	77.000	101.938	126.875	<b></b>	ļ
13' 0	Short		53.000	78.000	103.000	128.000		ļ
13' 0	Long		54.100	78.000	101.900	125.800 128.375	<b></b>	<b> </b>
13' 0	Bead		52.813 54.000	78.000	103.188	128.375	<b> </b>	<del> </del>
13' 2 13' 2	Short			79.000	104.000		<del>                                     </del>	
	Long		55.100	79.000	102.900	126.800 129.875		
13' 2 13' 4	Bead Short		53.563 54.400	79.000 80.000	104.438	131.200	ļ	
13' 4	Long	29,800		80.000	105.100	130.200		
13' 4	Bead	28.625		80.000	105,688	131.375		
13' 6	Short		55.400	81.000	106.600	132.200		
13' 6	Long		55.900	81,000	106,100	131.200		
13' 6	Bead	29.125		81.000	106.938	132.875		ļ
13' 8	Short		56.400	82.000	107.600	133.200		
13' 8	Long	31.250		82.000	107.375	132,750		
13' 8	Bead		55,813	82.000	108.188	134.375		
13' 10	Short	31.388	57.163	82.938	108.713	134,488		
13' 10	Long		57.170	83.000	108.830	134.660		
13' 10	Bead	30.125	56.563	83.000	109.438	135.875		
14' 0	Short	31.588	57.763	83.938	110.113	136.288		
14' 0	Long	33.250	58.625	84.000	109.375	134,750		L
14' 0	Bead		57.313	84.000	110.688	137.375		
14' 2	Short	32.754	58.846	84.938	111.413	137.888		
14' 2	Long	33.340	59.170	85.000	110.830	136.660		
14' 2	Bead	31.125	58.063	85.000	111.938	138.875		
14' 4	Short	32.388	59.163	85.938	112.713	139.488		
14' 4	Long	34.340	60.170	86.000	111.830	137.660		
14' 4	Bead	31.625	58.813	86.000	113.188	140.375		
14' 6	Short	32.788	59.863	86.938	114.013	141.088		
14' 6	Long	35.340		87.000	112.8300	138.660		
14' 6	Bead	32.125	59.563	87.000	114.438	141.875		
14' 8	Short	33.188		87.938	115.313	142.688		
14' 8	Long	23.218	44.812	66.406	88.000	109.594	131.188	152.78
14' 8	Bead	32.625	60.313	88.000	115.688	143.375		
14' 10	Short	33.588		88.938	116.613	144.288		
14' 10	Long	23.900		67.300	89.000		132,400	154.10
14' 10	Bead	33,125		89.000	116.938	144.875		
15' 0	Short		61.938	89,938	117.938	145.938		
15' 0	Long	24.900		68.300	90.000		133.400	155.10
15' 0	Bead	33.625		90,000	118.188	146.375		
15' 2	Short	34.388		90.938	119.213	147.488		
15' 2	Long	25.900		69,300	91.000		134.400	156.10
15' 2	Bead	34,125		91,000	119.438	147.875		
15' 4	Short	28.000		79.200	104.800		156.000	
15' 4	Long	24.875		69.625	92.000		136.750	159.12
15' 4	Bead	34.625		92,000	120.688	149.375		
15' 6	Short	23.436		69,812	93.000	116.188	139.376	162.56
15' 6	Long	24.900		70.300	93.000		138.400	
15' 6	Bead	24.626		70.209	93.000		138.583	
15' 8	***************************************	24.436		70.812	94,000		140.376	
	Short							
15' 8	Long	25.900		71.300	94.000		139.400	
15' 8	Bead	25.026		71.009	94.000		139.983	162.97
15' 10	Short	25.436		71.812	95.000		141.376	
15' 10	Long	26.251		72.084	95.000		140.833	
15' 10	Bead	23.426		71.809	95.000		141.383	
16' 0	Short	26.436		72.812	96.000		142.376	
16' 0	Long	27.900		73.300	96.000		141.400	
16' 0	Bead	24.626	48.417	72.209	96.000	119.792	143.583	167.37

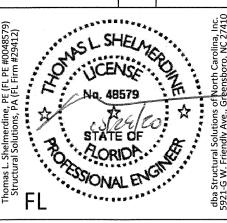
### TABLE 2

DOOR			SEC	MOIT	HEIC	SHTS	<del>e e per</del> apantante de la constante de la const		
HEIGHT	Btm	#2	#3	#4	#5	#6	#7	#8	
14' 0''	21"	21"	21"	21"	21"	21"	21"	21"	
13' 6''	21"	21"	21"	21"	21"	18"	18"	21"	
13' 0''	21"	21"	21"	18"	18"	18"	18"	21"	
12' 6''	21"	18"	18"	18"	18"	18"	18"	21"	
12' 0"	21"	21"	21"	21"	21"	18'	21"		
11' 6''	21"	21"	21"	18"	18"	18"	21"		
11' 0"	21"	18"	18"	18"	18"	18"	21"		
10' 6''	21"	21"	21"	21"	21"	21"			
10' 0''	21"	21"	21"	18"	18"	21"			
9'6''	21"	18"	18"	18"	18"	21"			
9' 0''	18"	18"	18"	18"	18"	18			
8' 6"	21"	21"	21"	18"	21"				
8' 0''	21"	18"	18"	18"	21"				-
7' 6"	18"	18"	18"	18"	18"				
7' 0"	21"	21"	21"	21"					
6' 6''	21"	18"	18"	21"					

REV	DESCRIPTION OF REVISIONS	DATE	BY
A	ADDED STRUT TO BOTTOM SECTION	02/04/13	RLR

DESIGN LOADS +45.8 PSF -49.3 PSF

LARGE MISSLE
IMPACT
RESISTANCE
Lyouns I. Shelm

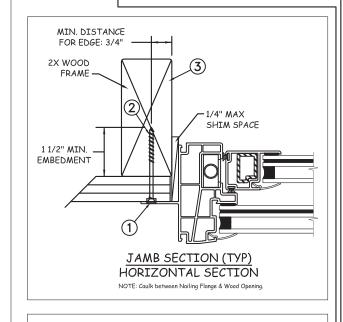


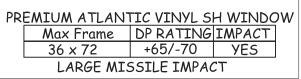


MODEL 950 AMARR HERITAGE (24 GA) 1000, 2000 MODEL 655 AMARR OAK SUMMIT (24 GA) 1000, 2000 SHORT, LONG, FLUSH, AND OAK SUMMIT PANELS

SIZE	DRAWN BY DL	DATE	03/12/03	Drawing Number			
В	CHECKED BY AA	DATE	03/14/03	IRC-9516-169-26			
165	EN CARRIAGE COURT V	SHEET 3 OF 3					

### NAIL FIN INSTALLATION





### -1 1/2" MIN. EMBEDMENT MIN. DISTANCE 1/4" MAX SHIM SPACE GLAZING DETAIL FRAME SECTION (TYP) VERTICAL SECTION SHIM SPACE MIN. DISTANCE FOR EDGE: 3/4" 1 1/2" MIN. EMBEDMENT

### Installation Notes:

1. Seal flange/frame to substrate.

HEIGHT (72"

2. Use #10 PH or greater fastener though the nail fin with sufficient length to penetrate a minimum of 1 1/2" into the wood framing. For 2X wood frame substrate (min, S.G. = 0.42).

WINDOW WIDTH (36" MAX.)

TYPICAL ELEVATION WITH FASTENER SPACING

Host structure (wood buck, masonry, steel) to be designed and anchored to properly transfer all loads to the structure. The host structure is the responsibility of the architect or engineer of record for the project of installation.

Digitally signed by Hermes F. Norero, P.E. Reason: I am approving this document Date: 2014.09.19 16:26:14 -04'00'

FROM CORNERS

This schedule addresses only the fasteners required to anchor the window to achieve the rated design pressure up to the size limitations noted. It is not intended as a guide to the installation process and does not address he sealing consideration that may arise in different wall conditions. For the complete installation procedure, see the instructions packaged with the window or go to www.ield-wen.com/resources/installation.

#### DISCLAIMER:

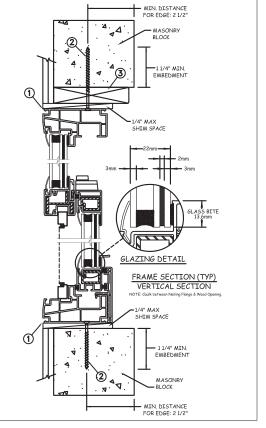
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### **General Notes:**

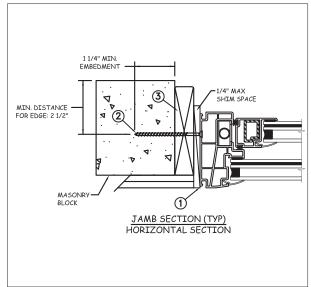
- The product shown herein is designed, tested and manufactured to comply with the wind load criteria
  of the adopted International Building Code(IBC), the International Residential Code(IRC), the Florida
  Building Code (FBC) including HVHZ and the industry requirement for the stated conditions.
- 2. All glazing shall conform to ASTM E1300.
- At minimum, glazing is 3mm annealed 11mm airspace 3mm annealed 2mm PVB Interlayer by DuPont - 3mm annealed.
- 4. Use structural or composite shims where required.

_	WESE NO								
	LIII 2ME	PROJECT ENGINEER:	DATE: 08/1	.3/2014	TET	DWE!	T		akeport Blvd
	10 70778	D. Vezo	SCALE:	NTS	JEL	LES VV C.I.			s, OR. 97601 11) 882-3451
	* * * *	CHECKED BY: J. Kantola	TITLE:	D	^_!	Vinyl Impact Sin	-l- 11	- \A/!	-l
	STATE OF STATE	APPROVED BY:		g wind	wor				
	HERMES NORERRO, P.E.	PART/PROJECT No.: D008729							
	398 East Damio Reach Elvd., Shite 33 Denia Beach, FL 33004	NCTL210-3875-1	PLANT NAM L-FBC	ME AND LOCAT	ION:	CAD DWG, No.:	REV: 00	SHEET	1 OF 3

# 22 6" MAX. FROM CORNERS WINDOW WIDTH (36" MAX.) TYPICAL ELEVATION WITH FASTENER SPACING



### MASONRY INSTALLATION



PF	REMIUM ATLANTIC VINYL SH WINDOW						
	Max Frame	DP RATING	IMPACT				
	36 x 72	+65/-70	YES				
LARGE MISSILE IMPACT							

#### Installation Notes:

- Seal flange/frame to substrate.
- Use 3/16" ITW Tapcon or equivalent fasteners through frame with sufficient length to penetrate a minimum of 1 1/4" into concrete or masonry at each location with a 2 1/2" min from edge distance. For concrete (min. f'c = 3000psi) or masonry substrate (CMU shall adhere to ASTM C90).
- Host structure (wood buck, masonry, steel) to be designed and anchored to properly transfer all loads to the structure. The host structure is the responsibility of the architect or engineer of record for the project of installation.

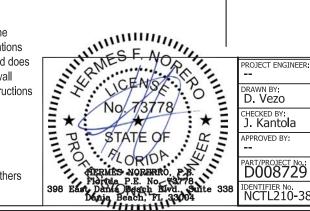
This schedule addresses only the fasteners required to anchor the window to achieve the rated design pressure up to the size limitations noted. It is not intended as a guide to the installation process and does not address he sealing consideration that may arise in different wall conditions. For the complete installation procedure, see the instructions packaged with the window or go to www.jeld-wen.com/resources/installation.

#### DISCLAIMER:

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#### **General Notes:**

- The product shown herein is designed, tested and manufactured to comply with the wind load criteria of the adopted International Building Code(IBC), the International Residential Code(IRC), the Florida Building Code (FBC) including HVHZ and the industry requirement for the stated conditions.
- All glazing shall conform to ASTM E1300.
- At minimum, glazing is 3mm annealed 11mm airspace 3mm annealed 2mm PVB Interlayer by DuPont - 3mm annealed.
- Use structural or composite shims where required,



DATE: 08/13/2014 JELD WEN SCALE: NTS

3737 Lakeport Blvd Klamath Falls, OR. 97601 Phone: (541) 882-3451

Premium Atlantic Vinyl Impact Single Hung Window

PART/PROJECT No. 1

IDENTIFIER NO. PLANT NAME AND LOCATION: NCTL210-3875-1-FBC

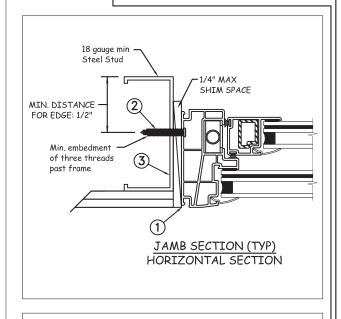
TITLE:

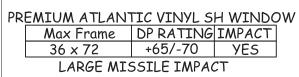
CAD DWG, No.:

00

2 OF 3

### STEEL INSTALLATION





### FOR EDGE: 1/2" Min. embedment of three threads 18 gauge min Steel Stud SHIM SPACE GLAZING DETAIL FRAME SECTION (TYP) VERTICAL SECTION 1/4" MAX SHIM SPACE - 18 aauae mir Steel Stud of three threads MIN. DISTANCE

#### Installation Notes:

- Seal flange/frame to substrate.
- For anchoring into metal framing use #8 TEK Self Tapping screws with sufficient length to achieve a
  minimum embedment of three threads past the frame thickness. Steel substrate min. 18ga., fy = 33 ksi.

WINDOW WIDTH (36" MAX.)

TYPICAL ELEVATION WITH FASTENER SPACING

5.5" MAX. FROM CORNERS

> 6" MAX. FROM CORNERS

Host structure (wood buck, masonry, steel) to be designed and anchored to properly transfer all loads
to the structure. The host structure is the responsibility of the architect or engineer of record for the
project of installation.

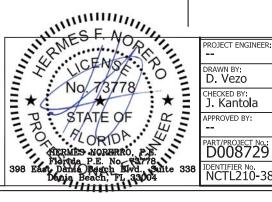
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### **General Notes:**

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- 2. All glazing shall conform to ASTM E1300.
- At minimum, glazing is 3mm annealed 11mm airspace 3mm annealed 2mm PVB Interlayer by DuPont - 3mm annealed.
- 4. Use structural or composite shims where required.



ROJECT ENGINEER:	DATE: 08/	13/2014	TET DAVEN	3737 Lakeport Blvd	
RAWN BY: D. Vezo	SCALE:	NTS	JELB-WEN	Klamath Falls, OR. 97601 Phone: (541) 882-3451	
HECKED BY: <b>Kantola</b>	TITLE:				
PPROVED BY:		Premiu	m Atlantic Vinyl Impact Single	ingle Hung Window	
ART/PROJECT No.: 0008729	1				

IDENTIFIER NO. PLANT NAME AND LOCATION: CAD DWG, No.: NCTL210-3875-1-FBC

v: 00 |<sup>sh</sup>

3 OF 3

### Appendix B:

Maps of the Subject Property



### **Community Planning and Preservation Commission**

2051 Burlington Ave N

AREA TO BE APPROVED,

**SHOWN IN** 

OVED, CASE NUMBER 22-90200063



